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LABORATORY STUDIES OF DEPTH DETERMINATION
OF THE WAVE VELOCITY METHOD

BY

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Berkeley, California
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I. ABSTRACT

The wave velocity method of depth determination has been studied in a laboratory wave channel. The data were recorded on movie film and were evaluated for wave travel diagrams (distance-time diagrams) which were then used to make depth predictions. Three sets of conditions were considered; 1) uniform waves in water of constant depth (not used for depth determination), 2) non-uniform waves in water of constant depth and 3) non-uniform waves on a uniformly sloping beach. The computed results were tabulated and the predicted depth obtained by averaging a large number of individual values. The resulting data are presented in a series of graphs.

II. INTRODUCTION

For military operations on enemy-held beaches, it is very important to know the water depths and characteristics of the coast line. Large scale hydrographic surveys of beach areas are rare and the areas covered usually are small. Furthermore, changes in the beach form can take place quite rapidly.^{(5)*} It is obvious that direct measurements of enemy-held beaches are very difficult. Some measurements have been made by sending boats or swimmers ashore at night to make the soundings, but this is a very dangerous task, and furthermore, there are no means of knowing exactly where the measurements were made. Although improvements have been made recently, during World War II considerable attention was given to the problem of depth determination, and different methods were developed to make indirect measurements of beach profiles. This report deals with the wave velocity method of depth determination. Several publications have described the application of this method (1-4). Photographs have been taken of a large variety of types of beaches and evaluated for depths. The results have been satisfactory insofar as the beach slopes were concerned; however, it was found that relatively large errors have occurred in determining depths at individual points. The difficulties in comparing the computed depths with actually measured depths may sometimes be traced back to the fact that the surf conditions often prevented soundings from being taken on the same day as the aerial sorties, and the actual profile may have changed considerably between the time of taking the pictures and the time that the soundings were completed.

Considering these difficulties, and also to obtain a larger variety of beach and wave conditions, it was decided to conduct some laboratory investigations on the wave velocity method of depth determination. Four sets of conditions were considered:

1. The velocities of uniform waves in constant depth of water were measured at different distances from the wave generator in order to determine the wave tank characteristics.
2. Non-uniform waves in water of constant depth, for several depths.

* Numbers in parentheses refer to References at end of paper.

3. Non-uniform waves over a beach of a constant slope of 1:40. This slope was chosen so as to be similar to field conditions previously analyzed. (9)
4. Non-uniform waves over beaches with slopes of 1:11, 1:20 and 1:40. Each of these beaches was used under two conditions, a) uniformly sloping without an offshore bar, and b) uniformly sloping with an offshore bar.

Evaluations of the data for the first three sets of conditions have been completed and are presented in this report. The results and evaluation of the fourth condition, however, will be presented in a separate report.

III. THEORY

If waves are of small amplitude compared to their length (height less than 1/200 of length) and depth of water, and are of constant length and height, the wave velocity C , in water of constant depth may be written as:

$$C^2 = (g L / 2\pi) \tanh (2\pi d/L) \quad (1)$$

or, considering also the definition for periodic waves

$$L = C T \quad (2)$$

Equation (1) may be rewritten as

$$C = (g T / 2\pi) \tanh (2\pi d/L) \quad (1a)$$

$$\text{or} \quad L = (g T^2 / \pi) \tanh (2\pi d/L) \quad (1b)$$

where C = wave velocity in feet per second,
 L = wave length in feet (the distance from crest to crest),
 T = wave period in seconds (the time interval between the appearance of successive crests at the same point),
 d = water depth in feet, and
 g = acceleration of gravity in feet/second².

Equation (1) indicates that the wave velocity and length depend upon the depth of water and the wave period. Considering also Equation (2), it is seen that the depth can be found if:

1. the velocity C and length L are known (Equation 1),
2. the velocity C and period T are known (Equation 1a), and
3. the length L and period T are known (Equation 1b).

If the depth is very shallow as compared with the wave length, $\tanh (2\pi d/L)$ approaches the value of $(2\pi d/L)$ and Equation (1) becomes

$$C^2 = gd \quad (3)$$

As we see in Figure 1, the two functions $(2\pi d/L)$ and $\tanh (2\pi d/L)$ are nearly equal for $d/L = 0.04$, or less.

At $d/L = 0.05$, the difference is 3% and at $d/L = 0.04$ the difference is only 2%. In other words, for $d/L = 0.04$ the wave length (and period) practically cease to influence the wave velocity and the depth may be determined by measuring only the wave velocities (Equation 3). For $T = 10$ seconds, the simplified Equation (3) may be used for depths approximately 5 feet or less, and $T = 15$ seconds when depths are less than 12 feet.

For water deeper than one-half the wave length ($d/L > 0.5$) $\tanh 2\pi d/L$ is almost equal to 1 and Equation (1) reduces to

$$C_o^2 = (g L_o / 2\pi) \quad (4)$$

$$C_o = (g T / 2\pi) \quad (4a)$$

Subscript $-o$ refers here to deep water conditions. In Equation (4) the wave velocity, length and period are not functions of the water depth, and so can not be used for depth determination.

For $d/L > 0.025$ the change in wave velocity seems to be rather insensitive to the change of depth, as can be seen in Figure 1, so it would be reasonable to limit the method in the laboratory studies to the region $d/L < 0.25$ (or the corresponding $d/L_o < 0.23$). To compare the laboratory studies with prototype conditions, we know that in many localities, such as the North Sea and the Baltic, and also wherever the locality is near a storm area, wave periods of 4 seconds may be encountered. This means that the method may be used for depths of less than 19 feet, which seems to be sufficient for most landing operations.

For $T = 3$ seconds, the method seems to be rather limited to a depth of approximately 10 feet. Considering also a relatively large error for higher d/L values, it seems to be reasonable to limit the method for wave periods $T \geq 4$ seconds. The above named limits are set considering only the results obtained under laboratory conditions. The actual application of the method has shown, however, that no satisfactory results may be obtained for such high values of d/L . For practical purposes $d/L = 0.1$ may be considered as the upper limit. On many occasions it was found that for satisfactory results the wave period T should be longer than 12 seconds. Wave crests of waves of shorter periods are scarcely distinguishable in aerial photographs.

As was previously mentioned, the wave velocity method has been applied on numerous occasions. The slopes of the beaches have been predicted satisfactorily in most cases; however, relatively large errors have been noted in the depths predicted at individual points. These errors can not all be traced to the accuracy of measurements; it seems rather that the theory breaks down upon occasions. To understand and minimize this difficulty it seems to be necessary to list and keep firmly in mind the assumptions and definitions that have led to Stokes' theory and Equation (1). The assumptions are as follows:

1. the water depth is constant,
2. the wave steepness is small, and
3. the waves are periodic and of uniform permanent form.

At first glance it appears that none of these conditions is fulfilled in the ocean. The first assumption (that the water depth should be constant) is almost never fulfilled when the theory is used to determine the depths

offshore from beaches, because the bottom is almost always sloping. Some of wave energy is reflected by the sloping bottom, but all the experiments indicate that reflections are very small and negligible when the slope is flatter than 1:10. The fact that the gradients of beaches were predicted very closely for flat beaches indicates also that the average wave velocities as predicted by the Stokes theory are not affected very much by sloping beaches.

The second assumption (that the wave steepness is small) is usually fulfilled in the ocean. However, it is preferable to use steeper waves, since the definition of the crest is always much better for steeper waves than for flat ones, and the error due to error in measurement will be reduced. Equation (1), known as Stokes' first approximation, neglects the effect of wave height. Stokes' third approximation takes this into account, that is,

$$C^2 = (g L / 2\pi) \tanh(2\pi d / L) \left\{ \frac{1 + e^{(8\pi d / L)} + e^{(-8\pi d / L)} + 2(e^{(4\pi d / L)} e^{(-4\pi d / L)}) + 12 \cdot \frac{\pi^2 H^2}{L^2}}{(e^{(2\pi d / L)} - e^{(-2\pi d / L)})^4} \right\} \quad (5)$$

where L , C and d are as defined before, and H = wave height.

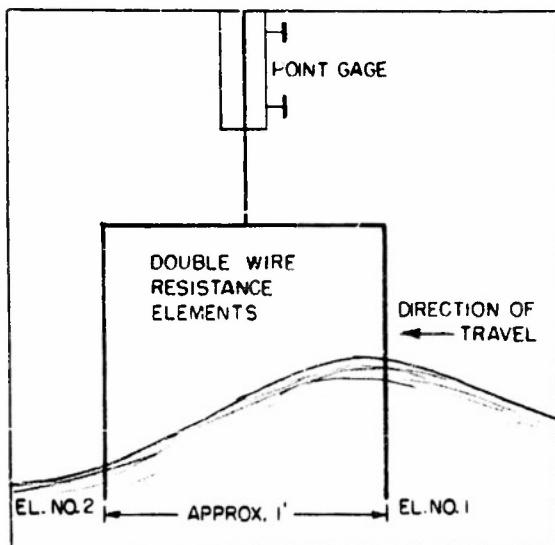
This formula, however, can not be used for depth determination, since it is practically impossible to measure wave heights from aerial photographs taken under operational conditions. However, one can estimate the relative wave steepness for a given photograph, and it has been the practice (particularly in Great Britain) to reduce the depths computed by 10 percent if the photographs indicate very steep waves, or by 5 percent for flatter waves (4).

Violation of the assumption that the waves are uniform seems to be the most critical. Ocean waves are always non-uniform, with the waves having different periods, lengths and heights. Thus wave forms are constantly changing due to their dispersive qualities; however, the changes become much less rapid in the shallow water near the beach, as this quality depends upon the relative depth, i.e., d/L .

IV. LABORATORY EQUIPMENT AND PROCEDURE

Experiments were performed in a wave channel 1 foot wide, 60 feet long and 3 feet deep, located in the Fluid Mechanics Laboratory of the University of California, Berkeley. One side of the channel consisted of plate-glass, framed in 3 ft. x 3 ft. steel frames, through which moving pictures of the wave motion were taken. The wave generator, of the flapper type, was located at one end of the channel. Both the amplitude and period of the flapper movement were adjustable. The period of the flapper movement could be varied between approximately 0.4 second and 2 seconds. At the opposite end of the channel from the wave generator an aluminum beach was installed. The slope of the beach could be varied from the horizontal to approximately 1:10.

To measure wave velocities at a particular point, two double-wire elements were mounted on a single point gage, as shown in the sketch. The distance between the two elements was approximately 1 foot. The elements were connected to a Brush Recorder and the surface time history recorded simultaneously for both elements (7). The time required for a wave crest to pass from element 1 to element 2 can be read very accurately from the records, and knowing the exact distance between the elements one is able



to compute the wave velocity. A sample of the record is given in Fig. 3a.

The first experiments were made to determine the influence of the particular channel upon the wave motion, and to determine the minimum distance between the wave-generator and the point of measurement that is required for steady conditions. For this purpose constant depths and uniform waves were used. The wave velocities were measured at different distances from the wave generator (see Figure 2).

For the remaining experiments, non-uniform waves were generated by moving the flapper manually in order to vary the periods and amplitudes. To obtain a continuous record of the waves a 35 mm Bell and Howell "Eymo" camera was used. The section covered by the camera was between 7 and 9 feet in length.

Experimentation with photography showed that clear water gave a surface line in the photographs which was difficult to read. Therefore, it was decided to cover the glass windows in the channel with tracing paper, stretched tightly against the surface of the glass. A grid was drawn on the paper to obtain a scale for the evaluation of the data. When a strong light was directed to the water surface a very clearly distinguished shadow line was obtained on the tracing paper; hence, the shadow of the water-surface profile was actually photographed. The results were satisfactory. Special care was taken to keep the tracing paper as tight as possible to the glass, otherwise the shadow image would not be clear and would result in erroneous readings. In order to obtain a time-scale for the measurements an electrically operated clock, graduated in 0.01 second increments, was installed in the field of view. The arrangement of the set-up is shown in Figure 4.

V. EVALUATION OF THE DATA

Uniform waves

The wave velocities for the uniform waves were obtained from the Brush records by measuring the time necessary for a wave crest to travel the known distance (approximately 1 foot) between the two resistance elements. The accuracy of the time measurements may be considered to be $\pm 2/1000$ of a second, provided that the definition of the wave crest was good, as is the case, for example, in Figure 3a.

In many cases it is very difficult to determine the exact location of a wave crest. The maximum elevation is not necessarily always in the middle of the wave, and in addition to that it seems to shift back and forth. The change in wave shape might be considerable even at such a short distance as 1 foot, as demonstrated clearly in Figure 3b. At element 1 the maximum elevation was at the front of the wave, but upon reaching element 2 it had already shifted considerably backward. It was obvious that the maximums in

Figure 3b could not be used to determine the wave velocity and that such data should be disregarded. The wave velocities were obtained for a carefully selected series of waves and averages were computed for at least ten waves. The measured wave velocities were compared with theoretical velocities as computed by Equation (1). Results are shown in Figure 2 as the ratio of C_m/C_c against the distance from the wave generator, measured in wave length D/L . Here C_m is the measured wave velocity, C_c the theoretical velocity, D the distance of measurement from the wave generator in feet and L the wave length.

Non-uniform waves

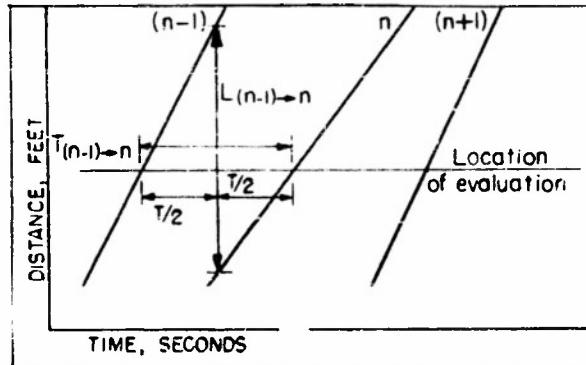
For the non-uniform waves it was necessary to obtain a continuous record of the wave travel, so the waves were photographed on 35 mm movie film as described above. The movies were analyzed frame by frame, each wave in the photographs being assigned a number, and the time and location of these waves were measured as they advanced down the channel. The data obtained were plotted as wave travel diagrams (see Figures 5, 10, 15, 20, 25 and 30.)

To compute the water depth, the wave velocities, periods and lengths were measured from wave travel diagrams. The wave lengths and periods were the distances between the successive wave crests in distance and time scale, respectively. The velocities were obtained by measuring the slopes of the time-distance curves. For the case of a constant depth of water, it was relatively easy to obtain the slope of the curve, since the curves could be represented by straight lines through the experimental points. The lines intercepted sometimes only through the caps, and shifts in lines (for example, in Figure 5, wave No. 18; and Figure 20, waves number 10, 12 and 16) were caused by the transformation of the waves and shifts in the position of wave crests. Much of the difficulty was experienced in obtaining the wave velocities for a sloping beach. The lines representing the travel of wave crests were curved, but the change in curvature was very small as the wave approached the beach. The velocity measurements had to be made as accurately as possible, since a relatively small error in velocity might cause a large error in depth. For the case of shallow water, where Equation (3) can be used, the error in depth would be the square of the error in velocity, as can easily be seen from this equation. It was found that the most reliable measurements of the curved lines representing the travel of wave crests were obtained by approaching the curve with a transparent ruler or triangle from the concave side until the straight edge established the best estimated tangent to this point; thus the velocity was obtained from as large as possible right triangle formed by the tangent and the length and time ordinates. The final value for the velocity was obtained as an average of at least five independent measurements at the given point. The measured values are tabulated in the first part of Tables 1, 2 etc.

Before starting the computations, it was necessary to decide which combination of C , T and L values should be used in Equation (1) for depth determination. There were many possibilities, such as using: (a) the preceding period and length to the crest where the velocity was measured, (b) the following period and length and (c) some combination of the preceding and following wave periods and lengths. To demonstrate the different possibilities for computation and to determine which of the methods was most

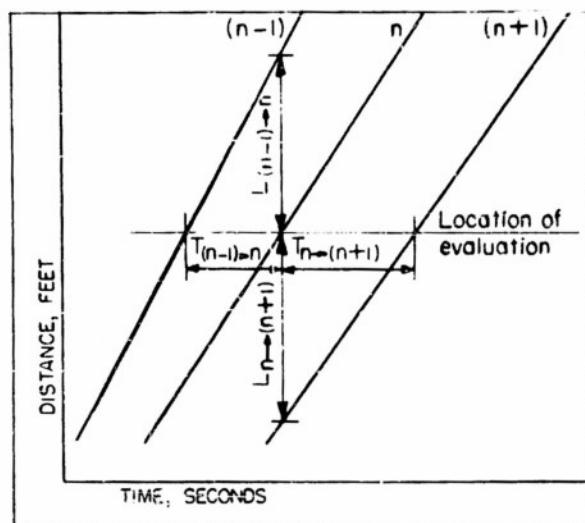
reliable and simple to handle, it was decided to use three different methods to evaluate Equation (1) for the depth. For comparison the simplified Equation (3) also was used for depth determination. Here it was necessary to measure only the crest velocity, and the method was introduced as Method 4. The other three methods used were as follows.

Method 1:



The depths were computed using the following combinations: (a) wave period $T_{(n-1)} \rightarrow n$, wave length $L_{(n-1)} \rightarrow n$, wave velocity $C_{(n-1)}$ of the preceding crest; (b) wave period $T_{(n-1)} \rightarrow n$ and wave length $L_{(n-1)} \rightarrow n$ as under (a), but the velocity of the following crest C_n . Thus, two computations were made for each wave, as shown in Table 1, 3, etc., labeled Method 1. The final depth was obtained by averaging all the single results.

Method 2: For each crest velocity C_n an average wave period T_n and L_n were computed as follows:



$$T_n = \frac{T_{(n-1)} \rightarrow n + T_{n \rightarrow (n+1)}}{2} \quad (7)$$

$$L_n = \frac{L_{(n-1)} \rightarrow n + L_{n \rightarrow (n+1)}}{2} \quad (8)$$

The values C_n , T_n , and L_n were used in Equation (1); the computations were completed in Tables 2, 4 etc. under Method 2.

Method 3:

Method 2 is satisfactory for more or less uniform wave trains. To consider also the location of crest n as compared with $(n-1)$ and $(n+1)$ (non-uniformity of waves) Method 3 was introduced. In this method a balanced wave period T_{nb} and length L_{nb} (linearly interpolated values and effective at the location where the velocity C_n was measured) was computed as follows:

$$T_{nb} = \frac{2 T_{(n-1)} \rightarrow n \times T_{n \rightarrow (n+1)}}{T_{(n-1)} \rightarrow n + T_{n \rightarrow (n+1)}} \quad (9)$$

$$L_{nb} = \frac{2 L_{(n-1)} \rightarrow n \times L_{n \rightarrow (n+1)}}{L_{(n-1)} \rightarrow n + L_{n \rightarrow (n+1)}} \quad (10)$$

The values C_n , T_{nb} and L_{nb} were used in Equation (1). The computations were completed in Tables 2, 4 etc., under Method 3.

Method 4:

The only measurement necessary here was wave velocity, and the depth was

computed in Tables 2, 4 etc., under Method 4, using the simplified Equation (3):

$$d_c = \frac{C_n^2}{g}$$

The computation procedure was the same for Methods 1 to 3 inclusive. Rewriting Equation (1a) we have

$$\tanh(2\pi d/L) = C_n / 5.12 \quad T = C_n / C_o \quad (11)$$

C_n and T were measured, so the value for $\tanh(2\pi d/L)$ could be computed at once, and referring this value to existing tables⁽⁸⁾ the value for d/L was obtained. With L known, the depth was easily computed.

As an example, a wave given in Table 2 under Crest 3 will be considered. Under Method 2 it was found that the average period $T_n = 0.96$ second, average length $L_n = 5.45$ feet, and the velocity $C_n = 4.43$ feet/second. Therefore

$$C_n / 5.12 T_n = \tanh(2\pi d/L) \text{ was found to be } \frac{4.43}{5.12 \times 0.96} = 0.903$$

Referring this value to Wiegel's tables⁽⁸⁾ page 45, we obtained $d/L = 0.237$; knowing L_n we have $d_c = 0.237 \times 5.45 = 1.29$ feet. Knowing also the actual depth of water, $d_m = 2.00$ feet, the absolute error and the percentage of error are computed as shown in Table 2. The negative sign indicates that the depth was underestimated; the positive sign designates overestimated depths. The distribution of errors was plotted for different methods in Figures 6-9, 11-14, 16-19, 21-24, and 26-29.

The computed wave lengths $L_c = C_n T_n$ (C_n and T_n were measured in wave travel diagrams) and the measured wave lengths L_n also were compared in Table 1 etc. under general data, but no attempt was made to use them for the depth determination.

At first it seemed reasonable to use only that part of the data where the values of L_c and L_n were approximately the same, but later it was discovered that there was no definite relationship between the error in measured wave length, L_n , as compared with the computed value, L_c ; hence it was decided to use only the measured wave lengths, L_n , for the computations.

IV. RESULTS AND DISCUSSION

Uniform waves

The first set of experiments were made using uniform waves in order to determine the characteristics of the wave channel. The results are given in Figure 2 which shows a comparatively wide scatter of the data close to the wave generator. In this region the measured wave velocities, C_m , seem to be higher than the theoretical velocities, indicating that the waves need a certain time to become stabilized after they are generated by the flapper. At a distance of approximately three wave lengths from the generator, the wave velocity seemed to develop a constant value, with measured values approximately 5 percent less than the theoretical. There were not enough data available to make this statement more conclusive; however, considering other observations, it seemed reasonable to maintain the points of measurement as far as possible from the generator.

Non-uniform waves in water of constant depth

In the wave theory used, an assumption of a finite and constant depth of water is made and then extended to shoaling water by assuming the waves to have the same characteristics in water of any depth as they would have in water of the same, but constant depth. The second set of experiments were made to test the validity of theory in water of constant depth. The experiments were made in five different depths of water: 2 feet; 1.5 feet; 1 foot; 0.533 foot and 0.253 foot with corresponding average $d/L_{oav.}$ values of 0.41; 0.34; 0.33; 0.104 and 0.067. The wave travel diagrams are given in Figures 5, 10, 15, 20, and 25, and the computations completed in Tables 1 to 8.

Effect of methods of computation: As already mentioned, the computations were completed using four different methods (see page 7). Method 1 was tedious as the computations had to be repeated twice for each crest; also, there seemed to be considerably more scatter in the results than in other methods (see Figures 6, 11, 16, 21, and 26). Because of this, the method is not recommended.

Methods 2 and 3 seemed to be of equal value and the computations indicated the least scatter in results, as can be seen in Figures 7, 8, 12, 13, 17, 18, 22, 23, 27 and 28. Method 2, which was slightly simpler to handle, is recommended for the case where the wave travel diagrams indicate a more or less uniform train of waves. Method 3 should be used in case of highly non-uniform waves.

Method 4, as mentioned before, should be used only for the case where the d/L value is less than 0.04. To demonstrate this, all the computations were completed for the fourth method also, and the results can be compared with other methods in Figures 9, 14, 19, 24 and 29. The results present little scatter, but the depths are considerably underestimated. The greater the relative depth of water, the larger the error. At $d/L_{oav.} = 0.41$ (Figure 9) the average error is - 63 percent; at $d/L_{oav.} = 0.10$ (Figure 24) the error is - 34 percent; and at $d/L_{oav.} = 0.067$ the error has been reduced to - 14 percent. There are no experimental results available for smaller d/L_o values, but all the available data⁽⁹⁾ indicate a very good agreement for small d/L_o values. Consequently, this method is recommended for the region of $d/L \leq 0.04$. The advantages of this method are the simplicity of application and the fact that only one measurement - the wave velocity - is necessary.

Effect of relative water depth d/L : The lower the value of d/L , the less scatter in results, as can be seen in the error distribution plots. This is to be expected because the shallower the water, the more depth affects the wave velocity, and hence, the less the importance of phase shift and period of the non-uniform waves. On the other hand, for the higher d/L values, the velocity is rather insensitive to the depth, and the curve $\tanh(2\pi d/L)$ develops a flatter and flatter shape (see Figure 1), so that it is very difficult to select the proper value for d/L for experimental values of $\tanh(2\pi d/L) = C_n / 5.12 T_n$. This results in an uncertain determination of depth. Thus, it is recommended that those data be used which have the longest possible wave lengths.

Effect of non-uniformity of the waves: In addition to the variation of lengths and periods, there are the phenomena of;

- a) the disappearance and reappearance of wave crests (see Figure 5, Crests 18-19; Figure 30, Crest 50; etc.);
- b) the instability of the wave crests, so that it is not possible to describe the travel of the crest by a single, well-defined curve (Figure 20, Crest 10);
- c) the shift in the crest (Figure 20, Crests 12, 16 etc.);
- d) the crossing of different wave crests (Figure 30, Crests 21 and 22); and
- e) the breaking of waves.

All the above-named irregularities in wave shape and appearance seem to affect considerably the wave characteristics as described by Equation (1). To demonstrate this fact, the error in depth determination was plotted in the upper portion of the wave travel diagrams (Figures 5, 10, 15, 20 and 25) at the location of the wave where the computations were made. As the case of $\tanh(2\pi d/L) = C_n/5.12 T_n > 1$ is indeterminant, no values could be determined; the blank spots in the error graphs usually are because of this factor. Large errors in computations usually can be traced to some kind of irregularity in wave shape, as described above under a) to e). The error is large, not only for the computations where the velocities of the unstable crests were used, but also for the neighboring crests, even when the travel diagram indicated a perfect crest for this wave. This condition is very clearly demonstrated in Figure 5 at Crests 18-19, Figure 10 at Crests 8-9, and Figure 20 at Crests 10 and 12. The errors usually started a couple of crests before the unstable crest, increased to a maximum at the location of the unstable crest and decreased again after that.

Thus, it is recommended that ample data be obtained to assure a definite and unique interpretation of crest lines in wave travel diagrams before starting the procedure of depth determination. All crests which indicate instability in plots should be excluded from the computations. Furthermore, the crests preceding and following the unstable crests (even when they seem perfect) should be excluded.

Effect of wave steepness: There were not many data available from this phase of the experiments to show the effect of wave steepness on the results of depth determination. Only Run 4 was evaluated for wave heights, and the corresponding steepnesses were plotted for each wave in the upper portion of the wave travel diagram (Figure 20). However, additional experiments will be evaluated for this effect and the results will be given in a separate report. Comparing the errors in computation and the corresponding wave steepnesses for Crests 5 to 8 (Figure 20) (these crests being reasonably well defined and not surrounded by unstable crests), it can be seen that the smaller steepnesses have resulted in a higher percentage of error. The error being always negative for the given case, a decrease in error indicated a higher velocity of travel for steeper waves, as mentioned and demonstrated previously by Equation (5). Definite conclusions cannot be stated without the support of more data.

Non-uniform waves on a uniformly sloping beach

The third set of experiments was done with non-uniform waves over a uniformly sloping beach with a slope of 1:40. The wave travel diagram is shown in Figure 30, and the results as averaged from the data of 26 waves are plotted in Figure 31. The computations were completed for seven stations according to Methods 2 and 3, and were given in Tables 9 to 15. The distribution of error is given in Figures 32 to 45. In this experiment it was found that all the statements made for the non-uniform waves in water of constant depth were also valid for the non-uniform waves on a sloping beach.

Effect of methods of computation: Method 2 and 3 were found to be of equal value, with Method 2 resulting in slightly smaller depths. But this might be of only local importance and might not be true for different cases.

Effect of relative water depth: It appears that less scatter in predicted depths occurs for the shallower depths, as can be seen in Figures 32 to 39. Figures 40 to 44, which are for relatively shallow water, indicate an increasing degree of scatter again. This latter can be traced to the steadily increasing number of breakers as the depth decreases. Breaking waves can not be used very well for depth determination.

Effect of non-uniformity of waves: It is recommended here again to use data only where the wave travel diagrams indicate well defined unique crest lines. Comparing the results in Tables 8 to 14 with the wave travel diagram in Figure 30, one can see clearly that the high percentage of errors can often be traced to unstable or breaking waves. As an example, Crests 33 and 39 yield the highest error at Station 32. At Station 35 the highest error of +100 percent is encountered at Crest 40 which breaks at this station and crosses with Crest 39. Naturally a great number of large errors are due to the high d/L value (short wave lengths) where depth determination results in uncertain values.

Effect of wave steepness: The wave heights were not evaluated for this run and so nothing definite can be stated. It seems, however, that the depths were overestimated for the shallower portions of the water, which indicates higher wave velocities than predicted by Equation (1). We know that the wave steepness increases as the wave moves over a sloping beach, and so the steadily increasing wave steepness might be the reason for higher wave velocities, hence, overestimation of the depth. Thus, the remark on Page 4 regarding the reduction of computed depths for steep waves seems to be warranted.

VII. CONCLUSIONS

Depth determination by the wave velocity method gives satisfactory results for relatively shallow water in the laboratory, provided many measurements are made to give a good average. The computations are relatively easy to complete and do not require highly trained personnel. However, it is necessary for the operations to be supervised by a person having a good understanding of the mechanism of wave motion; it is his task to make the choice as to which data to use. The remainder of the analysis can be completed using tabular computation forms by almost any intelligent person with a minimum of training. The computations for this report were made by different persons without any special training; and it was found that there was no appreciable difference in results obtained by different individuals.

The most important part of the procedure is the proper choice of data. It is suggested the following points should be kept in mind in making the choice:

1. The definition of the wave crests in the wave travel diagram should be unique and without any shift or break in line.
2. The neighboring wave crests to unstable crests (even when perfect in shape) should be excluded from the computations.
3. Exclude all crests at or in the immediate neighborhood of the breaking point.
4. Waves of the longest wave lengths (or periods) should be used.
5. As much data as possible should be amassed and the final result obtained by averaging all of the single results at a given location. The more irregular the waves the more data that is necessary to obtain a satisfactory average prediction of depth.

Concerning the different methods of computation, it might be said that:

1. For more or less uniform waves, Method 2 may be used.
2. For highly non-uniform waves Method 3 seems to be the most reasonable.
3. Method 4, utilizing the equation $d = C^2/g$ may be used only where there is enough proof available that $d/L \leq 0.04$.

VIII. REFERENCES

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TABLE 1

DEPTH DETERMINATION											
RUN NO. 1											
STATION LOCATION WAVE CHANNEL DATE 8-18-62											
GENERAL DATA											
ORDR	PERIOD Secs	PERIOD Secs	TIME Length Secs	WAVE Length Secs	WAVE Length Secs	WAVE Velocity Secs	L ₁ M ₁ M ₂ M ₃ M ₄ M ₅ M ₆ M ₇ M ₈ M ₉ M ₁₀ M ₁₁ M ₁₂ M ₁₃ M ₁₄ M ₁₅ M ₁₆ M ₁₇ M ₁₈ M ₁₉ M ₂₀ M ₂₁ M ₂₂ M ₂₃ M ₂₄ M ₂₅ M ₂₆ M ₂₇ M ₂₈ M ₂₉ M ₃₀ M ₃₁ M ₃₂ M ₃₃ M ₃₄ M ₃₅ M ₃₆ M ₃₇ M ₃₈ M ₃₉ M ₄₀ M ₄₁ M ₄₂ M ₄₃ M ₄₄ M ₄₅ M ₄₆ M ₄₇ M ₄₈ M ₄₉ M ₅₀ M ₅₁ M ₅₂ M ₅₃ M ₅₄ M ₅₅ M ₅₆ M ₅₇ M ₅₈ M ₅₉ M ₆₀ M ₆₁ M ₆₂ M ₆₃ M ₆₄ M ₆₅ M ₆₆ M ₆₇ M ₆₈ M ₆₉ M ₇₀ M ₇₁ M ₇₂ M ₇₃ M ₇₄ M ₇₅ M ₇₆ M ₇₇ M ₇₈ M ₇₉ M ₈₀ M ₈₁ M ₈₂ M ₈₃ M ₈₄ M ₈₅ M ₈₆ M ₈₇ M ₈₈ M ₈₉ M ₉₀ M ₉₁ M ₉₂ M ₉₃ M ₉₄ M ₉₅ M ₉₆ M ₉₇ M ₉₈ M ₉₉ M ₁₀₀ M ₁₀₁ M ₁₀₂ M ₁₀₃ M ₁₀₄ M ₁₀₅ M ₁₀₆ M ₁₀₇ M ₁₀₈ M ₁₀₉ M ₁₁₀ M ₁₁₁ M ₁₁₂ M ₁₁₃ M ₁₁₄ M ₁₁₅ M ₁₁₆ M ₁₁₇ M ₁₁₈ M ₁₁₉ M ₁₂₀ M ₁₂₁ M ₁₂₂ M ₁₂₃ M ₁₂₄ M ₁₂₅ M ₁₂₆ M ₁₂₇ M ₁₂₈ M ₁₂₉ M ₁₃₀ M ₁₃₁ M ₁₃₂ M ₁₃₃ M ₁₃₄ M ₁₃₅ M ₁₃₆ M ₁₃₇ M ₁₃₈ M ₁₃₉ M ₁₄₀ M ₁₄₁ M ₁₄₂ M ₁₄₃ M ₁₄₄ M ₁₄₅ M ₁₄₆ M ₁₄₇ M ₁₄₈ M ₁₄₉ M ₁₅₀ M ₁₅₁ M ₁₅₂ M ₁₅₃ M ₁₅₄ M ₁₅₅ M ₁₅₆ M ₁₅₇ M ₁₅₈ M ₁₅₉ M ₁₆₀ M ₁₆₁ M ₁₆₂ M ₁₆₃ M ₁₆₄ M ₁₆₅ M ₁₆₆ M ₁₆₇ M 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DEPTH DETERMINATION

TABLE 4

CREST NO.	PERIOD (Secs) T ₁	PERIOD (Secs) T ₂	AVER. WAVE LENGTH L _w (ft)	WAVE LENGTH L _w (ft)	WAVE VELOCITY L _w (ft/sec)	L _w L _w (ft)	L _w L _w (ft)	L _w L _w (ft)	L _w L _w (ft)	METHOD 2			METHOD 3			METHOD 4			GENERAL									
										T ₁ L _w (ft)	T ₂ L _w (ft)	T ₁ L _w (ft)	T ₂ L _w (ft)	T ₁ L _w (ft)	T ₂ L _w (ft)	AVER. WAVE LENGTH L _w (ft)	CREST NO.											
2	0.94	0.94	0.90	3.27	4.12	4.75	4.945	4.00	1.47	4.1	0.985	0.3045	1.478	0.076	2.07	0.80	0.00	4.80	0.988	4.088	1.053	3.162	10.2	0.645	-0.427	-5.7	—	2
3	0.94	0.94	0.88	4.50	3.91	4.065	4.942	4.03	1.36	4.2	1.01	—	—	—	—	0.879	0.05	4.49	1.04	—	—	0.643	-0.457	-3.7	—	3		
4	0.95	0.95	0.88	3.96	3.70	3.665	4.76	4.25	1.17	4.3	1.00	—	—	—	—	0.894	0.056	4.45	1.055	—	—	0.705	-0.455	-4.5	—	4		
5	0.98	0.98	0.98	4.42	3.36	3.695	4.31	3.685	4.25	4.37	0.98	0.980	1.000	0.091	0.07	0.98	3.04	4.35	0.982	4.088	1.053	3.162	10.2	0.645	-0.427	-5.7	—	5
6	0.79	0.91	0.93	3.82	4.82	2.770	4.275	3.63	4.0	4.5	0.983	0.3785	1.489	-0.072	-0.80	0.048	0.372	4.32	0.990	4.088	1.053	3.162	10.2	0.645	-0.427	-5.7	—	6
7	0.91	1.00	0.98	4.08	4.61	4.235	4.72	4.50	5.09	4.98	0.967	0.3035	1.377	-0.123	-0.30	0.053	0.423	4.675	0.949	3.808	1.390	0.104	-0.6	0.438	0.608	-4.5	—	7
8	1.00	1.07	1.08	4.68	3.98	4.995	4.545	4.05	4.47	4.7	0.89	0.3058	0.988	0.639	0.42	1.13	0.22	2.84	0.865	0.070	0.02	-0.414	0.413	0.817	-3.7	—	8	
9	1.07	1.00	1.08	4.76	4.53	4.705	4.00	3.975	3.55	3.03	0.665	0.3702	0.982	0.780	0.470	0.98	3.053	20.5	0.868	3.808	1.053	3.162	10.2	0.645	-0.427	-5.7	—	9
10	1.09	0.96	0.98	3.94	3.74	3.88	4.38	4.308	10.8	3.06	0.980	0.3008	0.780	0.780	0.470	0.98	3.053	20.5	0.868	3.808	1.053	3.162	10.2	0.645	-0.427	-5.7	—	10
11	0.98	0.91	0.92	3.84	4.40	4.18	4.80	3.70	2.0	4.4	0.914	0.2972	1.016	-0.068	-0.31	0.90	1.10	4.6	0.914	3.875	1.053	0.087	-3.4	0.540	-0.951	-6.4	—	11
12	0.91	0.96	0.95	3.98	4.33	1.03	4.80	4.63	1.16	4.33	1.04	—	—	—	—	0.944	0.058	0.92	1.04	0.944	0.058	0.92	0.746	0.754	-0.5	—	12	
13	0.91	0.91	0.95	4.67	4.98	4.66	4.47	4.82	2.0	4.65	0.982	0.2894	1.167	-0.283	-0.22	0.944	0.058	4.63	0.828	3.880	1.053	3.162	10.2	0.645	-0.427	-5.7	—	13
14	0.91	0.90	0.98	3.97	4.17	4.09	4.98	4.16	1.93	4.11	—	—	—	—	0.988	0.057	4.36	1.112	—	—	0.730	0.170	2.13	—	14			
Avr	—	—	—	4.09	4.16	4.473	4.346	—	—	—	—	1.166	0.344	2.98	0.053	0.11	—	—	1.166	0.301	2.023	0.626	0.378	-0.6	—	Avr	—	—

DEPTH DETERMINATION

TABLE 5

CREST NO.	PERIOD (Secs) T ₁	PERIOD (Secs) T ₂	AVER. WAVE LENGTH L _w (ft)	WAVE LENGTH L _w (ft)	WAVE VELOCITY L _w (ft/sec)	L _w L _w (ft)	L _w L _w (ft)	L _w L _w (ft)	L _w L _w (ft)	METHOD 2			METHOD 3			METHOD 4			GENERAL									
										T ₁ L _w (ft)	T ₂ L _w (ft)	T ₁ L _w (ft)	T ₂ L _w (ft)	T ₁ L _w (ft)	T ₂ L _w (ft)	AVER. WAVE LENGTH L _w (ft)	CREST NO.											
1	0.72	0.70	0.71	2.79	3.08	3.80	3.67	3.085	1.19	3.70	1.047	—	—	—	—	0.730	0.085	3.70	1.060	—	—	0.690	-0.410	-5.0	—	2		
2	0.70	0.62	0.70	3.00	2.21	3.13	3.07	4.07	3.01	3.94	4.07	1.000	—	—	—	—	0.704	0.110	4.088	1.053	—	—	0.591	-0.448	-4.8	—	3	
3	0.92	0.70	0.70	3.40	3.10	3.20	3.94	3.108	1.15	4.08	0.980	0.3890	0.988	-0.088	-0.81	0.100	0.3240	4.088	0.988	0.088	0.813	0.468	-0.932	-4.8	—	4		
4	0.70	0.73	0.78	3.07	2.70	2.920	4.07	3.07	3.80	3.98	1.064	—	—	—	—	0.704	0.150	3.80	1.064	—	—	0.581	-0.488	-4.8	—	5		
5	0.73	0.71	0.72	3.00	2.06	2.94	3.70	2.71	-0.81	3.00	1.081	—	—	—	—	0.720	0.193	3.96	1.081	—	—	0.580	-0.481	-4.8	—	6		
6	0.71	0.75	0.76	2.71	3.14	3.080	3.97	2.96	1.05	3.64	1.084	—	—	—	—	0.740	0.175	3.83	1.084	—	—	0.580	-0.481	-4.8	—	7		
7	0.70	0.61	0.61	2.16	3.20	3.19	4.10	3.32	0.70	4.14	0.980	0.2840	1.263	-0.982	-0.29	0.343	3.19	4.14	0.980	0.2840	1.263	2.93	0.807	-0.983	-9.3	—	8	
8	0.91	0.61	0.61	3.30	3.16	3.16	4.10	3.32	0.70	4.14	0.980	0.2840	1.263	-0.982	-0.29	0.343	3.19	4.14	0.980	0.2840	1.263	2.93	0.807	-0.983	-9.3	—	9	
9	0.91	0.77	0.76	3.09	2.96	3.04	3.67	3.08	1.70	3.00	0.980	0.4784	1.167	-0.467	-0.47	0.47	0.710	3.04	3.00	0.980	0.4784	1.167	2.90	0.860	-0.910	-10	—	10
10	0.70	0.77	0.76	3.09	2.96	3.04	3.67	3.08	1.70	3.00	0.980	0.4784	1.167	-0.467	-0.47	0.47	0.710	3.04	3.00	0.980	0.4784	1.167	2.90	0.860	-0.910	-10	—	11
11	0.70	0.73	0.76	3.07	2.95	3.06	3.99	3.25	-1.12	3.21	1.041	—	—	—	—	0.700	0.195	3.21	1.041	—	—	0.580	-0.480	-4.8	—	12		
12	0.73	0.74	2.98	3.00	2.96	3.06	3.99	3.25	-1.12	3.21	1.041	—	—	—	—	0.700	0.195	3.21	1.041	—	—	0.580	-0.480	-4.8	—	13		
13	0.60	0.60	0.63	3.48	3.46	4.24	3.98	3.87	4.04	4.000	1.000	—	—	—	—	0.689	0.405	4.04	1.000	—	—	0.580	-0.440	-4.8	—	14		
14	0.65	0.76	3.47	2.98	3.16	4.17	3.30	2.13	4.14	1.003	—	—	—	—	0.687	0.315	4.17	1.009	—	—	0.580	-0.480	-4.8	—	15			
15	0.70	0.75	3.08	2.80	2.70	3.76	3.06	-0.01	3.768	0.988	0.684	0.1867	0.987	0.987	0.87	0.74	0.770	2.70	0.988	0.684	0.1867	0.987	2.73	0.479	-0.561	-17	—	16
16	0.71	0.82	2.92	2.92	3.01	3.27	3.06	-0.27	3.10	0.980	0.684	0.1867	0.987	0.987	0.87	0.74	0.770	2.73	0.988	0.684	0.1867	0.987	2.73	0.479	-0.561	-17	—	17
17	0.71	0.82	2.92	2.92	3.01	3.27	3.06	-0.27	3.10	0.980	0.684	0.1867	0.987	0.987	0.87	0.74	0.770	2.73	0.988	0.684	0.1867	0.987	2.73	0.479	-0.561	-17	—	18
18	0.65	0.76	2.13	2.77	3.46	3.79	3.06	-0.16	3.10	0.980	0.684	0.1867	0.987	0.987	0.87	0.74	0.770	2.73	0.988	0.684	0.1867	0.987	2.73	0.479	-0.561	-17	—	19
19	0.67	1.13	0.60	2.00	3.46	2.74	3.66																					

DEPTH DETERMINATION

TABLE 7

GENERAL DATA										METHOD 2										METHOD 3										METHOD 4										GENERAL																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																					
CREST NO.	PERIOD (Secs.)	PERIOD (Secs.)	AVG. LENGTH (ft.)	WAVE LENGTH (ft.)	AVG. LENGTH (ft.)	WAVE LENGTH (ft.)	VELOCITY (ft./sec.)	L₁ (ft.)	L₂ (ft.)	10⁻³ T₀ (ft.)	512 T₀ (ft.)	512 T₀ (ft.)	A₁	A₂	A₃	A₄	A₅	A₆	A₇	A₈	A₉	A₁₀	A₁₁	A₁₂	A₁₃	A₁₄	A₁₅	A₁₆	A₁₇	A₁₈	A₁₉	A₂₀	A₂₁	A₂₂	A₂₃	A₂₄	A₂₅	A₂₆	A₂₇	A₂₈	A₂₉	A₃₀	A₃₁	A₃₂	A₃₃	A₃₄	A₃₅	A₃₆	A₃₇	A₃₈	A₃₉	A₄₀	A₄₁	A₄₂	A₄₃	A₄₄	A₄₅	A₄₆	A₄₇	A₄₈	A₄₉	A₅₀	A₅₁	A₅₂	A₅₃	A₅₄	A₅₅	A₅₆	A₅₇	A₅₈	A₅₉	A₆₀	A₆₁	A₆₂	A₆₃	A₆₄	A₆₅	A₆₆	A₆₇	A₆₈	A₆₉	A₇₀	A₇₁	A₇₂	A₇₃	A₇₄	A₇₅	A₇₆	A₇₇	A₇₈	A₇₉	A₈₀	A₈₁	A₈₂	A₈₃	A₈₄	A₈₅	A₈₆	A₈₇	A₈₈	A₈₉	A₉₀	A₉₁	A₉₂	A₉₃	A₉₄	A₉₅	A₉₆	A₉₇	A₉₈	A₉₉	A₁₀₀	A₁₀₁	A₁₀₂	A₁₀₃	A₁₀₄	A₁₀₅	A₁₀₆	A₁₀₇	A₁₀₈	A₁₀₉	A₁₁₀	A₁₁₁	A₁₁₂	A₁₁₃	A₁₁₄	A₁₁₅	A₁₁₆	A₁₁₇	A₁₁₈	A₁₁₉	A₁₂₀	A₁₂₁	A₁₂₂	A₁₂₃	A₁₂₄	A₁₂₅	A₁₂₆	A₁₂₇	A₁₂₈	A₁₂₉	A₁₃₀	A₁₃₁	A₁₃₂	A₁₃₃	A₁₃₄	A₁₃₅	A₁₃₆	A₁₃₇	A₁₃₈	A₁₃₉	A₁₄₀	A₁₄₁	A₁₄₂	A₁₄₃	A₁₄₄	A₁₄₅	A₁₄₆	A₁₄₇	A₁₄₈	A₁₄₉	A₁₅₀	A₁₅₁	A₁₅₂	A₁₅₃	A₁₅₄	A₁₅₅	A₁₅₆	A₁₅₇	A₁₅₈	A₁₅₉	A₁₆₀	A₁₆₁	A₁₆₂	A₁₆₃	A₁₆₄	A₁₆₅	A₁₆₆	A₁₆₇	A₁₆₈	A₁₆₉	A₁₇₀	A₁₇₁	A₁₇₂	A₁₇₃	A₁₇₄	A₁₇₅	A₁₇₆	A₁₇₇	A₁₇₈	A₁₇₉	A₁₈₀	A₁₈₁	A₁₈₂	A₁₈₃	A₁₈₄	A₁₈₅	A₁₈₆	A₁₈₇	A₁₈₈	A₁₈₉	A₁₉₀	A₁₉₁	A₁₉₂	A₁₉₃	A₁₉₄	A₁₉₅	A₁₉₆	A₁₉₇	A₁₉₈	A₁₉₉	A₂₀₀	A₂₀₁	A₂₀₂	A₂₀₃	A₂₀₄	A₂₀₅	A₂₀₆	A₂₀₇	A₂₀₈	A₂₀₉	A₂₁₀	A₂₁₁	A₂₁₂	A₂₁₃	A₂₁₄	A₂₁₅	A₂₁₆	A₂₁₇	A₂₁₈	A₂₁₉	A₂₂₀	A₂₂₁	A₂₂₂	A₂₂₃	A₂₂₄	A₂₂₅	A₂₂₆	A₂₂₇	A₂₂₈	A₂₂₉	A₂₃₀	A₂₃₁	A₂₃₂	A₂₃₃	A₂₃₄	A₂₃₅	A₂₃₆	A₂₃₇	A₂₃₈	A₂₃₉	A₂₄₀	A₂₄₁	A₂₄₂	A₂₄₃	A₂₄₄	A₂₄₅	A₂₄₆	A₂₄₇	A₂₄₈	A₂₄₉	A₂₅₀	A₂₅₁	A₂₅₂	A₂₅₃	A₂₅₄	A₂₅₅	A₂₅₆	A₂₅₇	A₂₅₈	A₂₅₉	A₂₆₀	A₂₆₁	A₂₆₂	A₂₆₃	A₂₆₄	A₂₆₅	A₂₆₆	A₂₆₇	A₂₆₈	A₂₆₉	A₂₇₀	A₂₇₁	A₂₇₂	A₂₇₃	A₂₇₄	A₂₇₅	A₂₇₆	A₂₇₇	A₂₇₈	A₂₇₉	A₂₈₀	A₂₈₁	A₂₈₂	A₂₈₃	A₂₈₄	A₂₈₅	A₂₈₆	A₂₈₇	A₂₈₈	A₂₈₉	A₂₉₀	A₂₉₁	A₂₉₂	A₂₉₃	A₂₉₄	A₂₉₅	A₂₉₆	A₂₉₇	A₂₉₈	A₂₉₉	A₃₀₀	A₃₀₁	A₃₀₂	A₃₀₃	A₃₀₄	A₃₀₅	A₃₀₆	A₃₀₇	A₃₀₈	A₃₀₉	A₃₁₀	A₃₁₁	A₃₁₂	A₃₁₃	A₃₁₄	A₃₁₅	A₃₁₆	A₃₁₇	A₃₁₈	A₃₁₉	A₃₂₀	A₃₂₁	A₃₂₂	A₃₂₃	A₃₂₄	A₃₂₅	A₃₂₆	A₃₂₇	A₃₂₈	A₃₂₉	A₃₃₀	A₃₃₁	A₃₃₂	A₃₃₃	A₃₃₄	A₃₃₅	A₃₃₆	A₃₃₇	A₃₃₈	A₃₃₉	A₃₄₀	A₃₄₁	A₃₄₂	A₃₄₃	A₃₄₄	A₃₄₅	A₃₄₆	A₃₄₇	A₃₄₈	A₃₄₉	A₃₅₀	A₃₅₁	A₃₅₂	A₃₅₃	A₃₅₄	A₃₅₅	A₃₅₆	A₃₅₇	A₃₅₈	A₃₅₉	A₃₆₀	A₃₆₁	A₃₆₂	A₃₆₃	A₃₆₄	A₃₆₅	A₃₆₆	A₃₆₇	A₃₆₈	A₃₆₉	A₃₇₀	A₃₇₁	A₃₇₂	A₃₇₃	A₃₇₄	A₃₇₅	A₃₇₆	A₃₇₇	A₃₇₈	A₃₇₉	A₃₈₀	A₃₈₁	A₃₈₂	A₃₈₃	A₃₈₄	A₃₈₅	A₃₈₆	A₃₈₇	A₃₈₈	A₃₈₉	A₃₉₀	A₃₉₁	A₃₉₂	A₃₉₃	A₃₉₄	A₃₉₅	A₃₉₆	A₃₉₇	A₃₉₈	A₃₉₉	A₄₀₀	A₄₀₁	A₄₀₂	A₄₀₃	A₄₀₄	A₄₀₅	A₄₀₆	A₄₀₇	A₄₀₈	A₄₀₉	A₄₁₀	A₄₁₁	A₄₁₂	A₄₁₃	A₄₁₄	A₄₁₅	A₄₁₆	A₄₁₇	A₄₁₈	A₄₁₉	A₄₂₀	A₄₂₁	A₄₂₂	A₄₂₃	A₄₂₄	A₄₂₅	A₄₂₆	A₄₂₇	A₄₂₈	A₄₂₉	A₄₃₀	A₄₃₁	A₄₃₂	A₄₃₃	A₄₃₄	A₄₃₅	A₄₃₆	A₄₃₇	A₄₃₈	A₄₃₉	A₄₄₀	A₄₄₁	A₄₄₂	A₄₄₃	A₄₄₄	A₄₄₅	A₄₄₆	A₄₄₇	A₄₄₈	A₄₄₉	A₄₅₀	A₄₅₁	A₄₅₂	A₄₅₃	A₄₅₄	A₄₅₅	A₄₅₆	A₄₅₇	A₄₅₈	A₄₅₉	A₄₆₀	A₄₆₁	A₄₆₂	A₄₆₃	A₄₆₄	A₄₆₅	A₄₆₆	A₄₆₇	A₄₆₈	A₄₆₉	A₄₇₀	A₄₇₁	A₄₇₂	A₄₇₃	A₄₇₄	A₄₇₅	A₄₇₆	A₄₇₇	A₄₇₈	A₄₇₉	A₄₈₀	A₄₈₁	A₄₈₂	A₄₈₃	A₄₈₄	A₄₈₅	A₄₈₆	A₄₈₇	A₄₈₈	A₄₈₉	A₄₉₀	A₄₉₁	A₄₉₂	A₄₉₃	A₄₉₄	A₄₉₅	A₄₉₆	A₄₉₇	A₄₉₈	A₄₉₉	A₅₀₀	A₅₀₁	A₅₀₂	A₅₀₃	A₅₀₄	A₅₀₅	A₅₀₆	A₅₀₇	A₅₀₈	A₅₀₉	A₅₁₀	A₅₁₁	A₅₁₂	A₅₁₃	A₅₁₄	A₅₁₅	A₅₁₆	A₅₁₇	A₅₁₈	A₅₁₉	A₅₂₀	A₅₂₁	A₅₂₂	A₅₂₃	A₅₂₄	A₅₂₅	A₅₂₆	A₅₂₇	A₅₂₈	A₅₂₉	A₅₃₀	A₅₃₁	A₅₃₂	A₅₃₃	A₅₃₄	A₅₃₅	A₅₃₆	A₅₃₇	A₅₃₈	A₅₃₉	A₅₄₀	A₅₄₁	A₅₄₂	A₅₄₃	A₅₄₄	A₅₄₅	A₅₄₆	A₅₄₇	A₅₄₈	A₅₄₉	A₅₅₀	A₅₅₁	A₅₅₂	A₅₅₃	A₅₅₄	A₅₅₅	A₅₅₆	A₅₅₇	A₅₅₈	A₅₅₉	A₅₆₀	A₅₆₁	A₅₆₂	A₅₆₃	A₅₆₄	A₅₆₅	A₅₆₆	A₅₆₇	A₅₆₈	A₅₆₉	A₅₇₀	A₅₇₁	A₅₇₂	A₅₇₃	A₅₇₄	A₅₇₅	A₅₇₆	A₅₇₇	A₅₇₈	A₅₇₉	A₅₈₀	A₅₈₁	A₅₈₂	A₅₈₃	A₅₈₄	A₅₈₅	A₅₈₆	A₅₈₇	A₅₈₈	A₅₈₉	A₅₉₀	A₅₉₁	A₅₉₂	A₅₉₃	A₅₉₄	A₅₉₅	A₅₉₆	A₅₉₇	A₅₉₈	A₅₉₉	A₆₀₀	A₆₀₁	A₆₀₂	A₆₀₃	A₆₀₄	A₆₀₅	A₆₀₆	A₆₀₇	A₆₀₈	A₆₀₉	A₆₁₀	A₆₁₁	A₆₁₂	A₆₁₃	A₆₁₄	A₆₁₅	A₆₁₆	A₆₁₇	A₆₁₈	A₆₁₉	A₆₂₀	A₆₂₁	A₆₂₂	A₆₂₃	A₆₂₄	A₆₂₅	A₆₂₆	A₆₂₇	A₆₂₈	A₆₂₉	A₆₃₀	A₆₃₁	A₆₃₂	A₆₃₃	A₆₃₄	A₆₃₅	A₆₃₆	A₆₃₇	A₆₃₈	A₆₃₉	A₆₄₀	A₆₄₁	A₆₄₂	A₆₄₃	A₆₄₄	A₆₄₅	A₆₄₆	A₆₄₇	A₆₄₈	A₆₄₉	A₆₅₀	A₆₅₁	A₆₅₂	A₆₅₃	A₆₅₄	A₆₅₅	A₆₅₆	A₆₅₇	A₆₅₈	A₆₅₉	A₆₆₀	A₆₆₁	A₆₆₂	A₆₆₃	A₆₆₄	A₆₆₅	A₆₆₆	A₆₆₇	A₆₆₈	A₆₆₉	A₆₇₀	A₆₇₁	A₆₇₂	A₆₇₃	A₆₇₄	A₆₇₅	A₆₇₆	A₆₇₇	A₆₇₈	A₆₇₉	A₆₈₀	A₆₈₁	A₆₈₂	A₆₈₃	A₆₈₄	A₆₈₅	A₆₈₆	A₆₈₇	A₆₈₈	A₆₈₉	A₆₉₀	A₆₉₁	A₆₉₂	A₆₉₃	A₆₉₄	A₆₉₅	A₆₉₆	A₆₉₇	A₆₉₈	A₆₉₉	A₇₀₀	A₇₀₁	A₇₀₂	A₇₀₃	A₇₀₄	A₇₀₅	A₇₀₆	A₇₀₇	A₇₀₈	A₇₀₉	A₇₁₀	A₇₁₁	A₇₁₂	A₇₁₃	A₇₁₄	A₇₁₅	A₇₁₆	A₇₁₇	A₇₁₈	A₇₁₉	A₇₂₀	A₇₂₁	A₇₂₂	A₇₂₃	A₇₂₄	A₇₂₅	A₇₂₆	A₇₂₇	A₇₂₈	A₇₂₉	A₇₃₀	A₇₃₁	A₇₃₂	A₇₃₃	A₇₃₄	A₇₃₅	A₇₃₆	A₇₃₇	A₇₃₈	A₇₃₉	A₇₄₀	A₇₄₁	A₇₄₂	A₇₄₃	A₇₄₄	A₇₄₅	A₇₄₆	A₇₄₇	A₇₄₈	A₇₄₉	A₇₅₀	A₇₅₁	A₇₅₂	A₇₅₃	A₇₅₄	A₇₅₅	A₇₅₆	A₇₅₇	A₇₅₈	A₇₅₉	A₇₆₀	A₇₆₁	A₇₆₂	A₇₆₃	A₇₆₄	A₇₆₅	A₇₆₆	A₇₆₇	A₇₆₈	A₇₆₉	A₇₇₀	A₇₇₁	A₇₇₂	A₇₇₃	A₇₇₄	A₇₇₅	A₇₇₆	A₇₇₇	A₇₇₈	A₇₇₉	A₇₈₀	A₇₈₁	A₇₈₂	A₇₈₃	A₇₈₄	A₇₈₅	A₇₈₆	A₇₈₇	A₇₈₈	A₇₈₉	A₇₉₀	A₇₉₁	A₇₉₂	A₇₉₃	A₇₉₄	A₇₉₅	A₇₉₆	A₇₉₇	A₇₉₈	A₇₉₉	A₈₀₀	A₈₀₁	A₈₀₂	A₈₀₃	A₈₀₄	A₈₀₅	A₈₀₆	A₈₀₇	A₈₀₈	A₈₀₉	A₈₁₀	A₈₁₁	A₈₁₂	A₈₁₃	A₈₁₄	A₈₁₅	A₈₁₆	A₈₁₇	A₈₁₈	A₈₁₉	A₈₂₀	A₈₂₁	A₈₂₂	A₈₂₃	A₈₂₄	A₈₂₅	A₈₂₆	A₈₂₇	A₈₂₈	A₈₂₉	A₈₃₀	A₈₃₁	A₈₃₂	A₈₃₃	A₈₃₄	A₈₃₅	A₈₃₆	A₈₃₇	A₈₃₈	A₈₃₉	A₈₄₀	A₈₄₁	A₈₄₂	A₈₄₃	A₈₄₄	A₈₄₅	A₈₄₆	A₈₄₇	A₈₄₈	A<

DEPTH DETERMINATION

TABLE 9

DEPTH DETERMINATION

TABLE 10

DEPTH DETERMINATION

TABLE II

DEPTH DETERMINATION

TABLE 12

DEPTH DETERMINATION

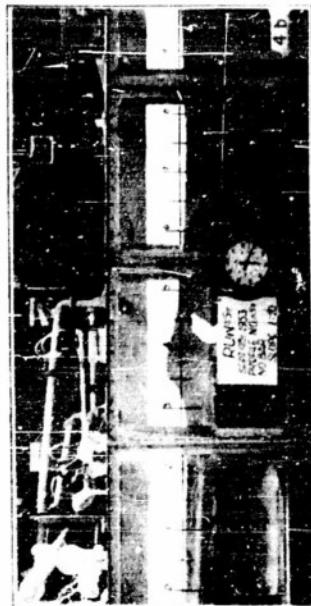
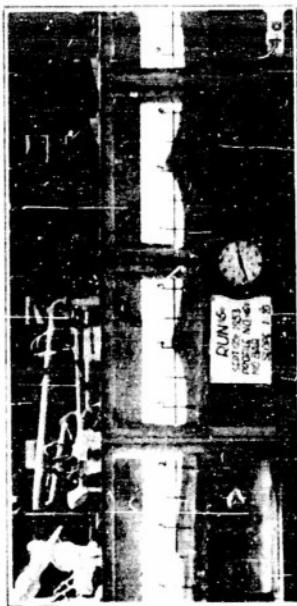
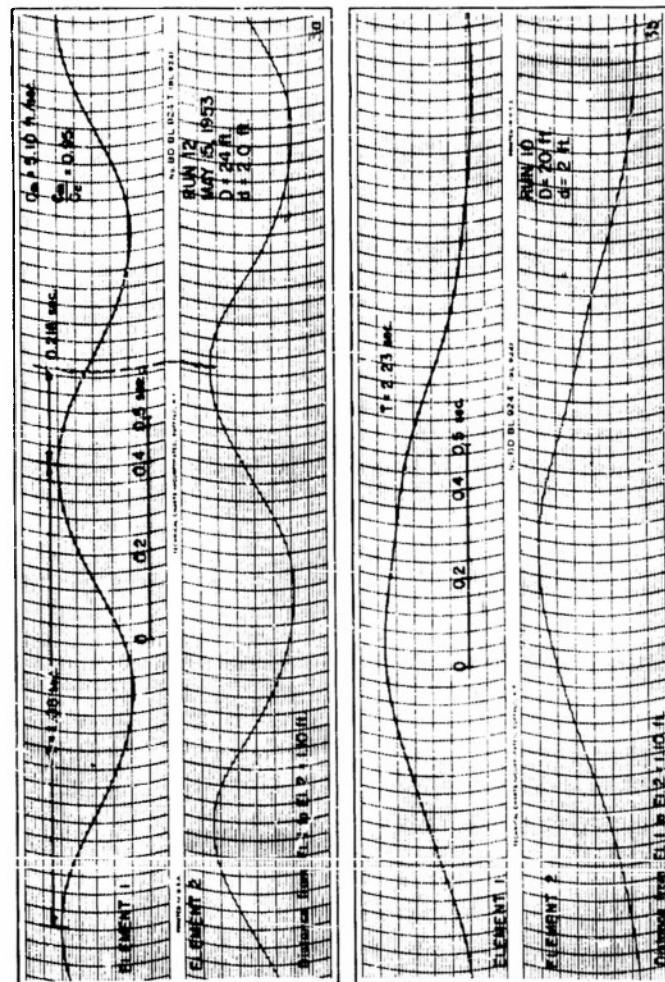
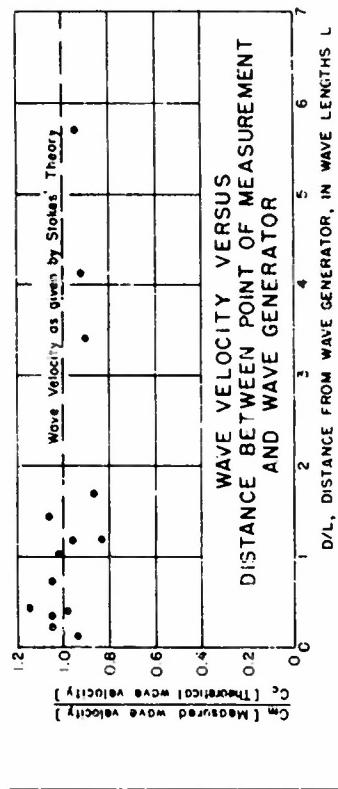
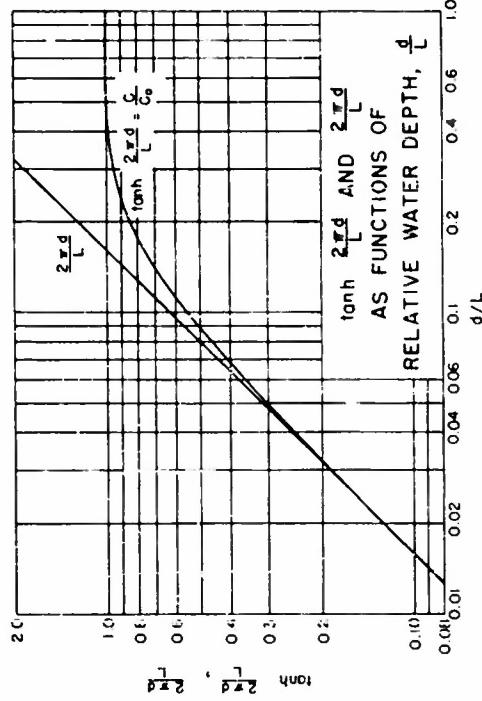
TABLE 15

RUN NO	—	WAVES NON-UNIFORM
STATION	47'	MEASURED DEPTH 4a 0.275 ft
LOCATION	WAVE CHANNEL	SLOPE 1.40
DATE	10-23-52	PROFILE UNIFORM SLOPE

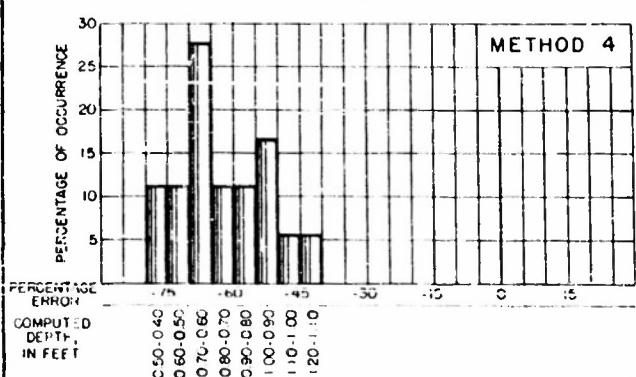
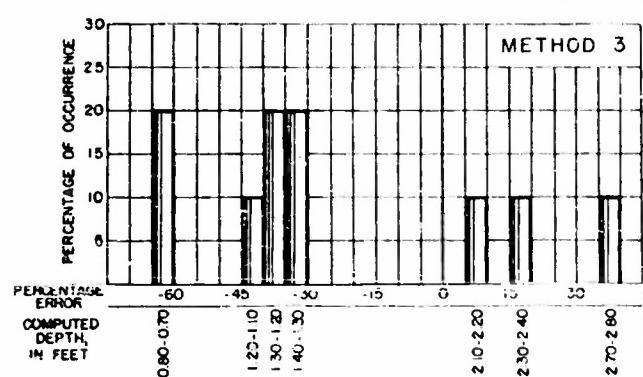
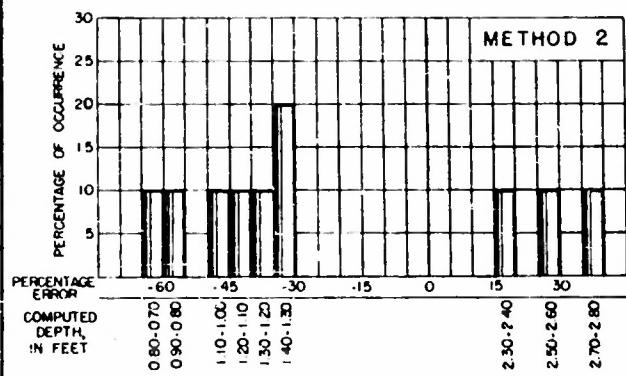
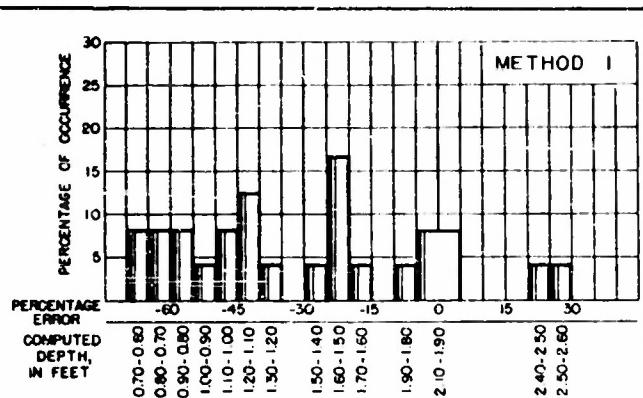
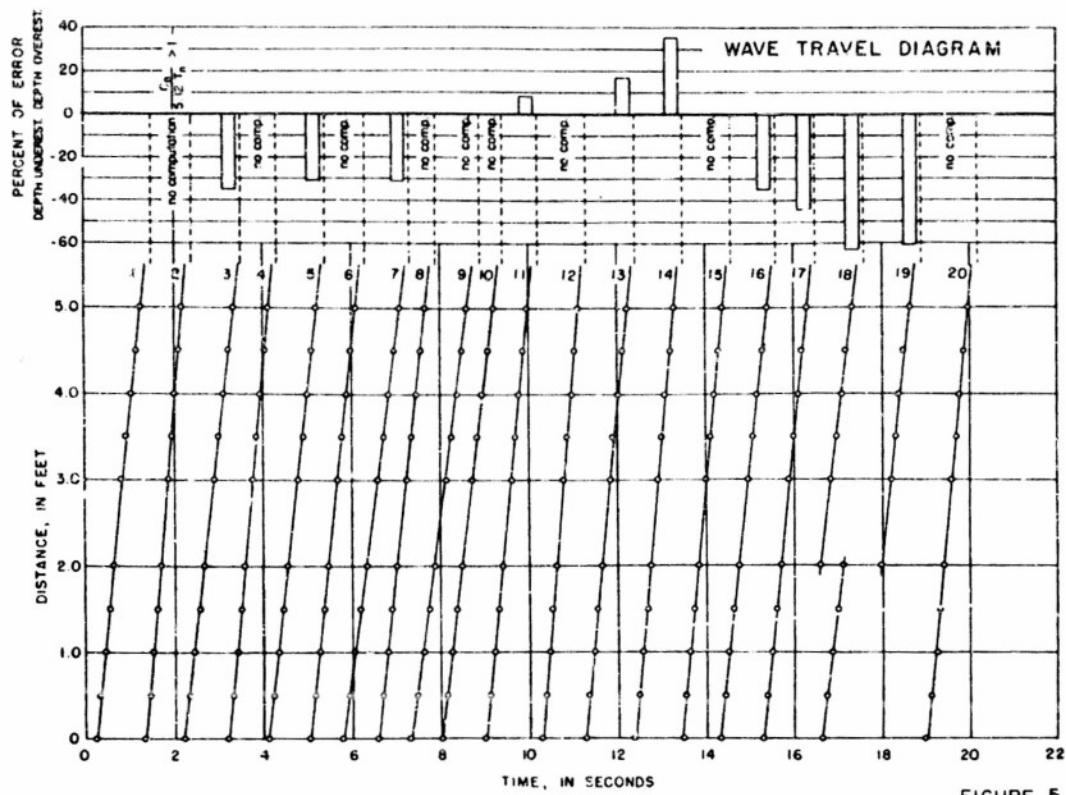
DEPTH DETERMINATION

TABLE 14

RUN NO	50'	WAVES NON-UNIFORM
STATION	WAVE CHANNEL	MEASURED DEPTH = 0.200 ft
LOCATION		SLOPE 1:40
DATE	10-23-82	PROFILEZ UNIFORM SLOPE



A sample of 35mm movies taken of a section of wave channel for the purpose of depth determination by wave velocity method



RUN 1

Non-uniform waves in constant depth

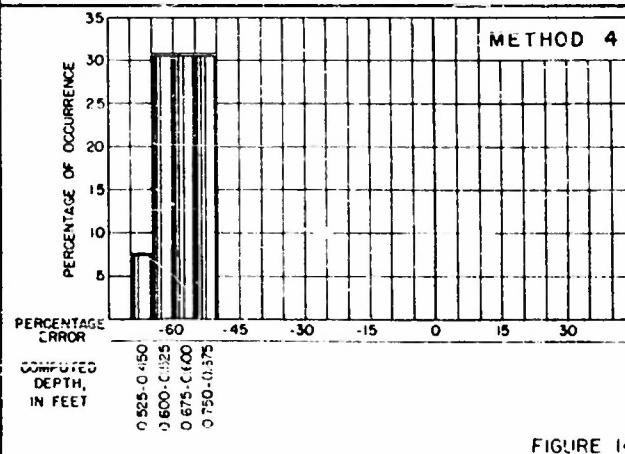
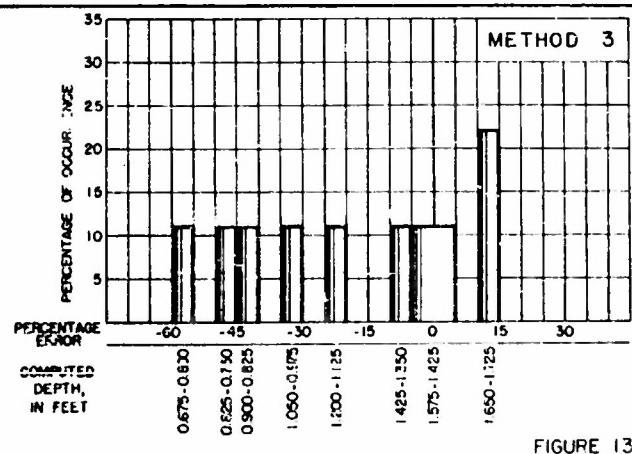
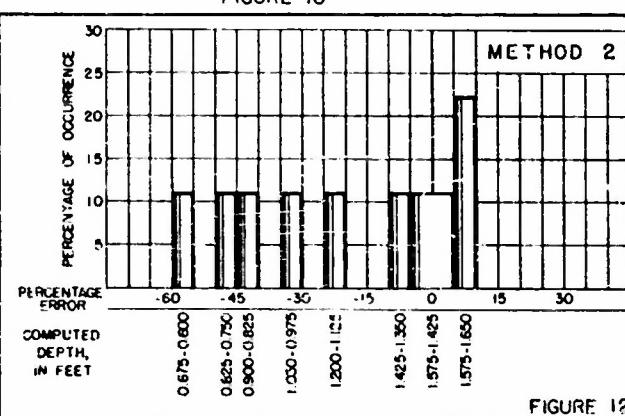
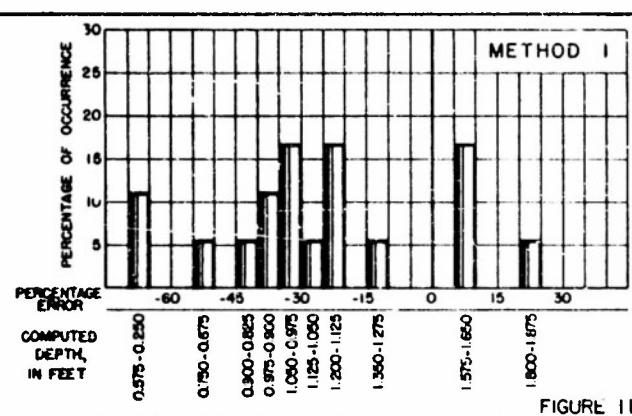
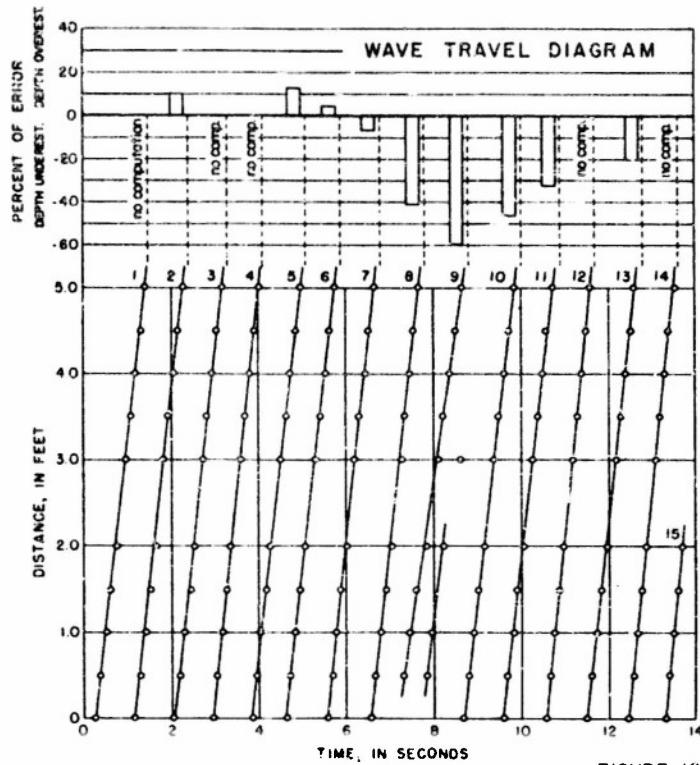
Depth of water $d = 2.00$ feet

$d/L_{ave} = 0.414$

FIGURE 9

HYD 6757

FIGURES 5 - 9



RUN 2

Non-uniform waves in constant depth

Depth of water $d = 1.50$ feet

$d/L_{wave} = 0.34$

FIGURES 10 - 14

HYD-6759

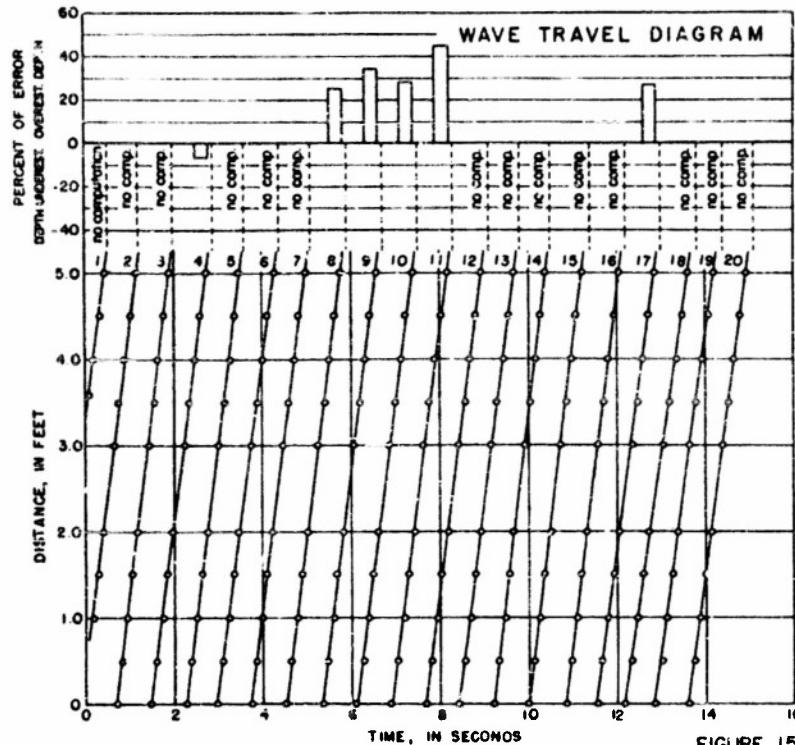


FIGURE 15

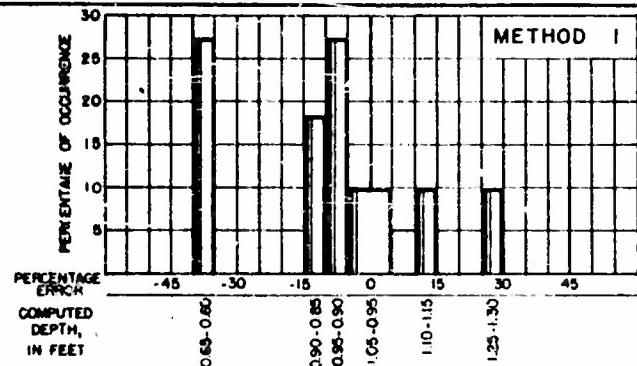


FIGURE 16

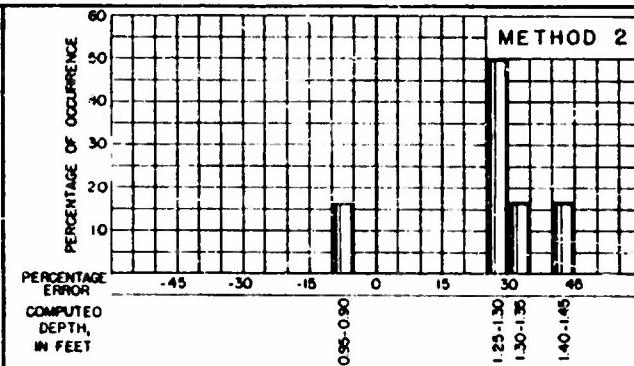


FIGURE 17

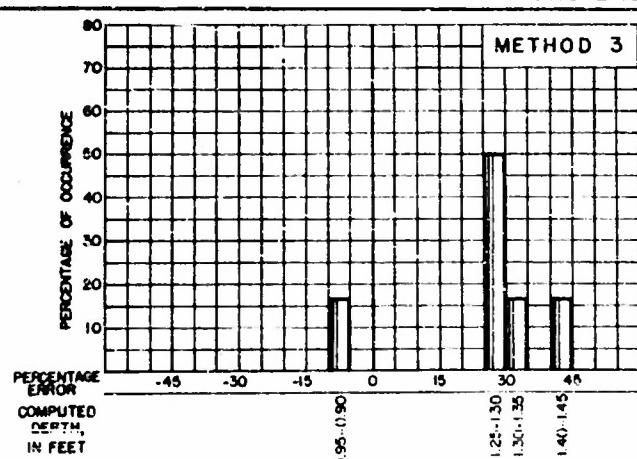


FIGURE 18

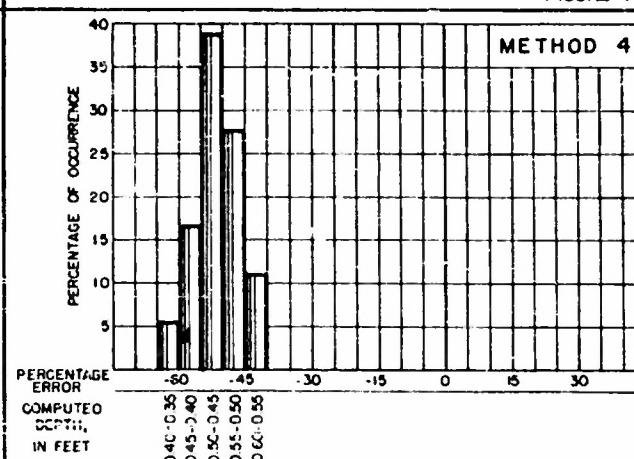


FIGURE 19

HYD-6759

RUN 3

Non-uniform waves in constant depth

Depth of water $d = 1.00$ foot

$d/L_{0,ave} = 0.335$

FIGURES 15 - 19

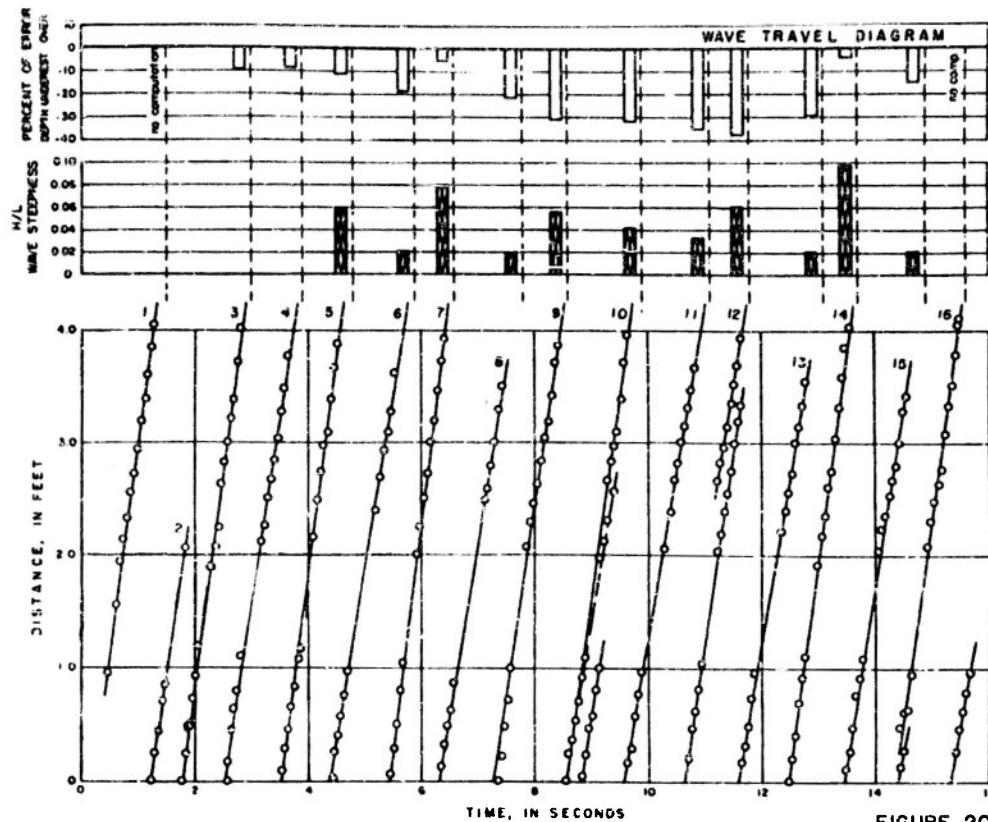


FIGURE 20

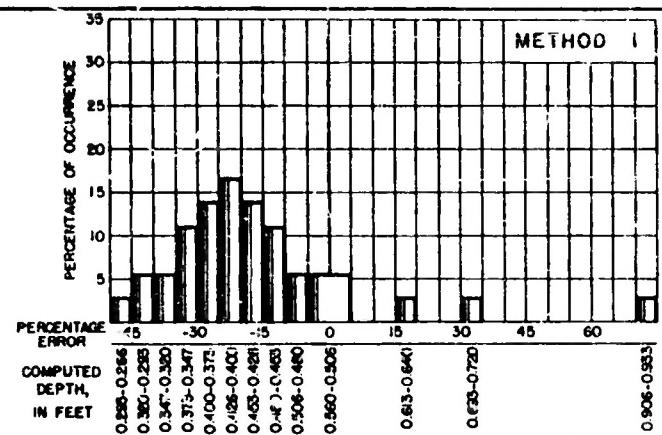


FIGURE 21

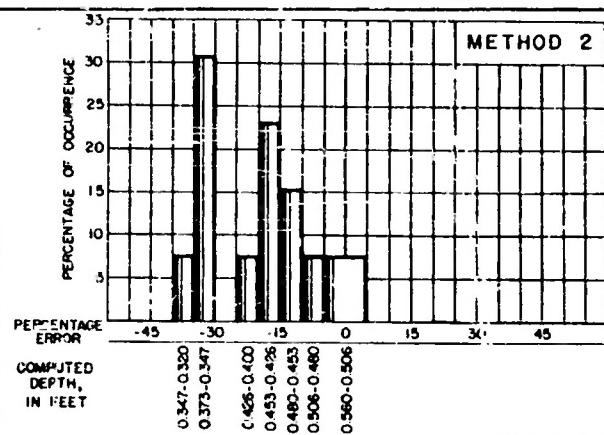


FIGURE 22

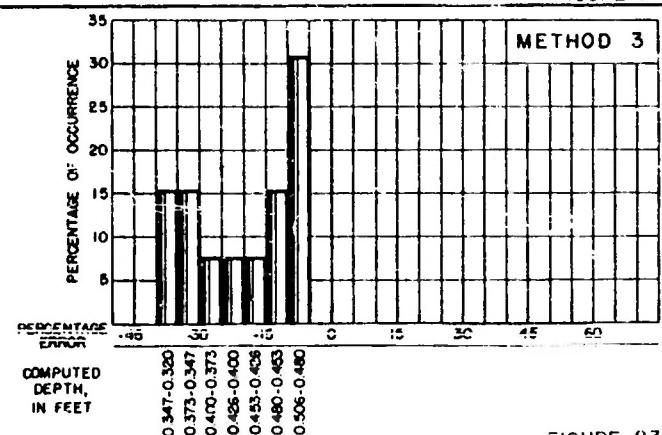


FIGURE 23

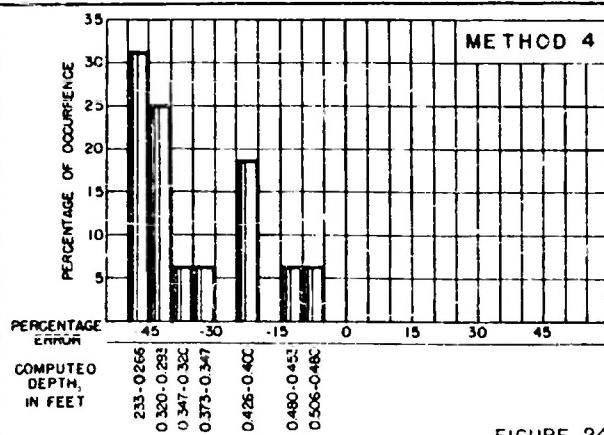


FIGURE 24

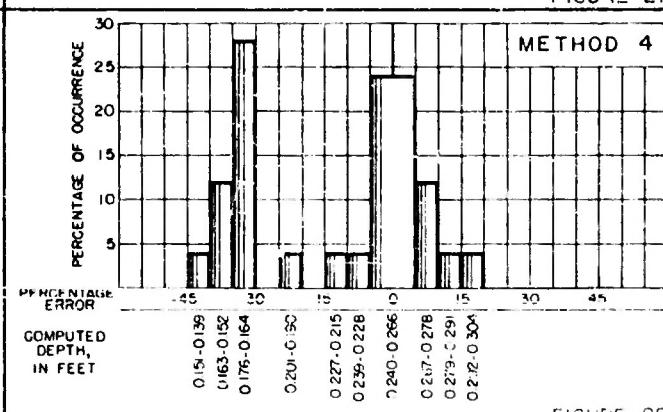
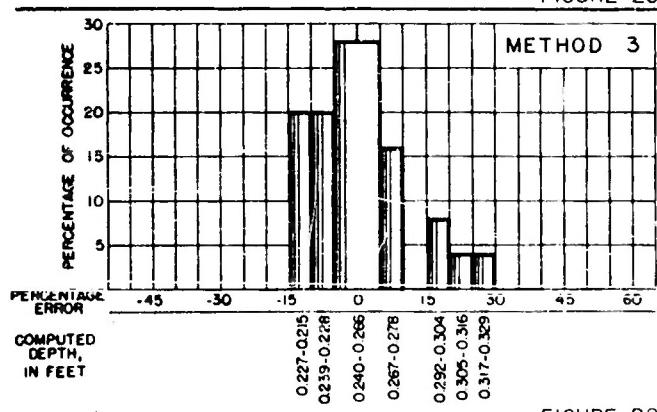
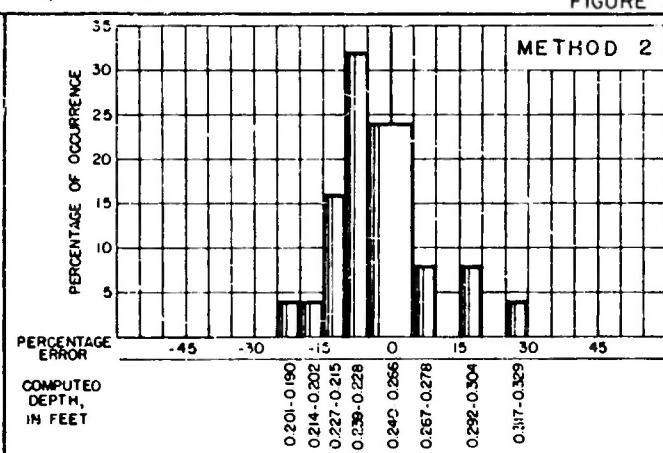
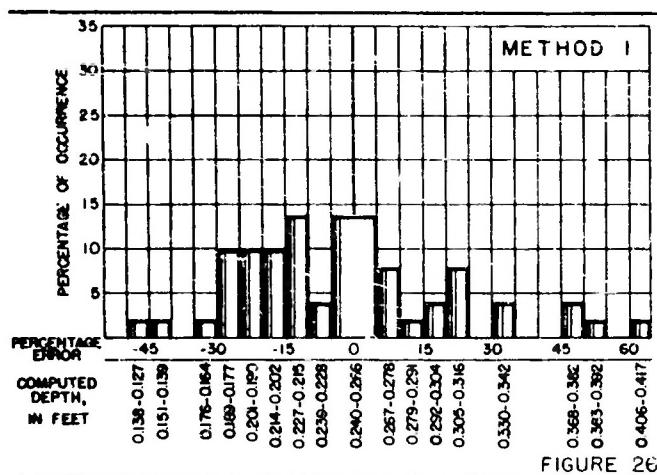
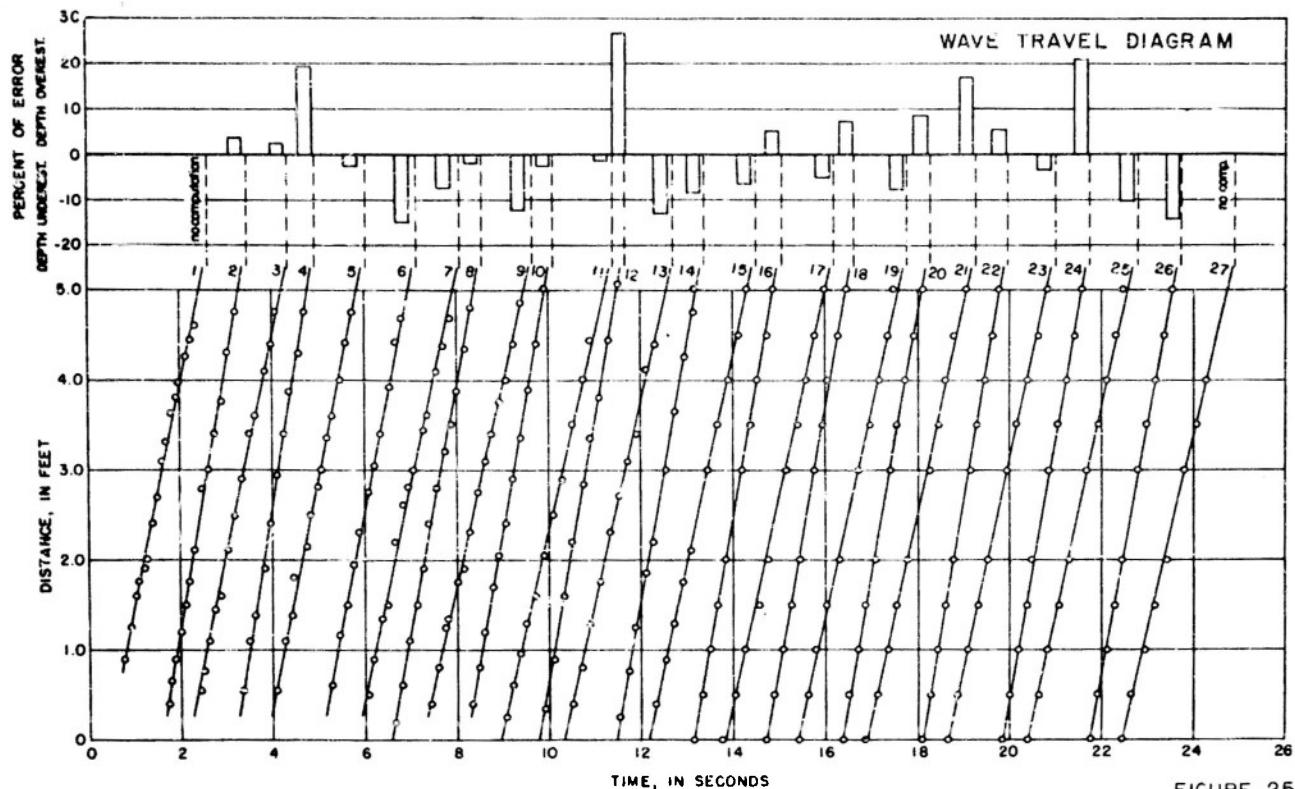
RUN 4

Non-uniform waves in constant depth

Depth of water $d = 0.533$ foot, $d/L_{0,ave} = 0.104$

HYD-6760

FIGURES 20-24



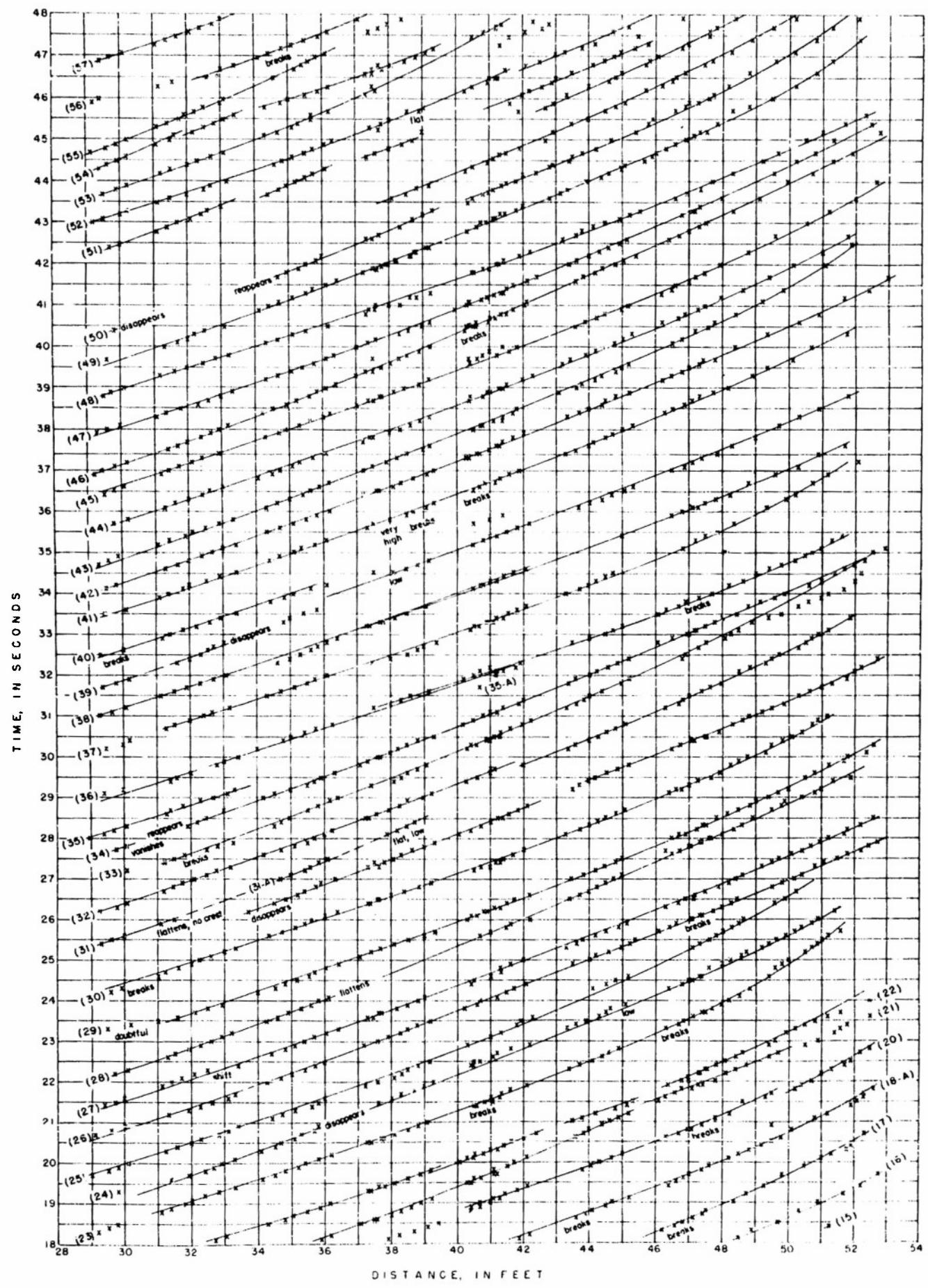
RUN 5

Non-uniform waves in constant depth

Depth of water $d = 0.253$ foot

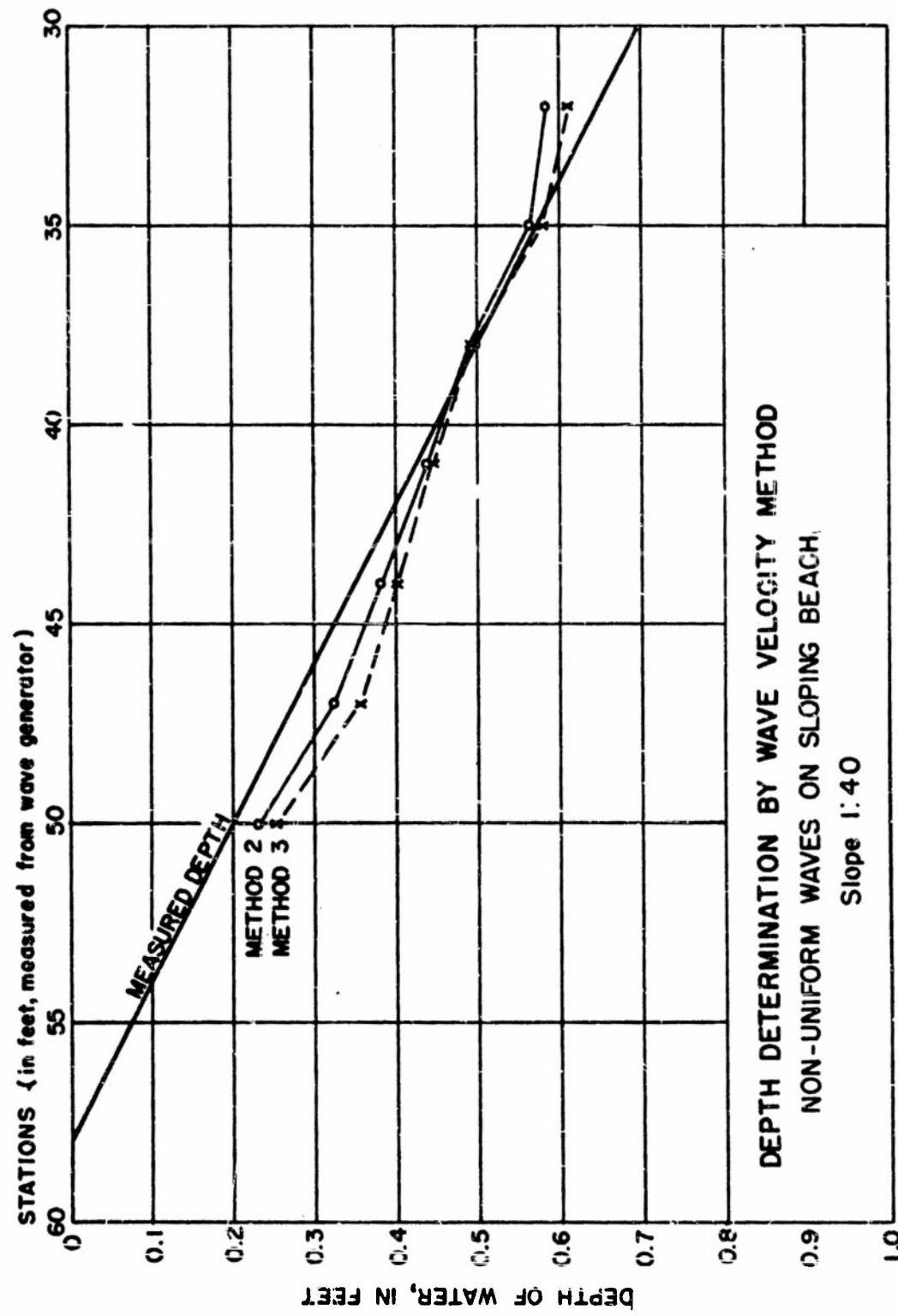
$d/L_{ave} = 0.067$

FIGURES 25 - 29



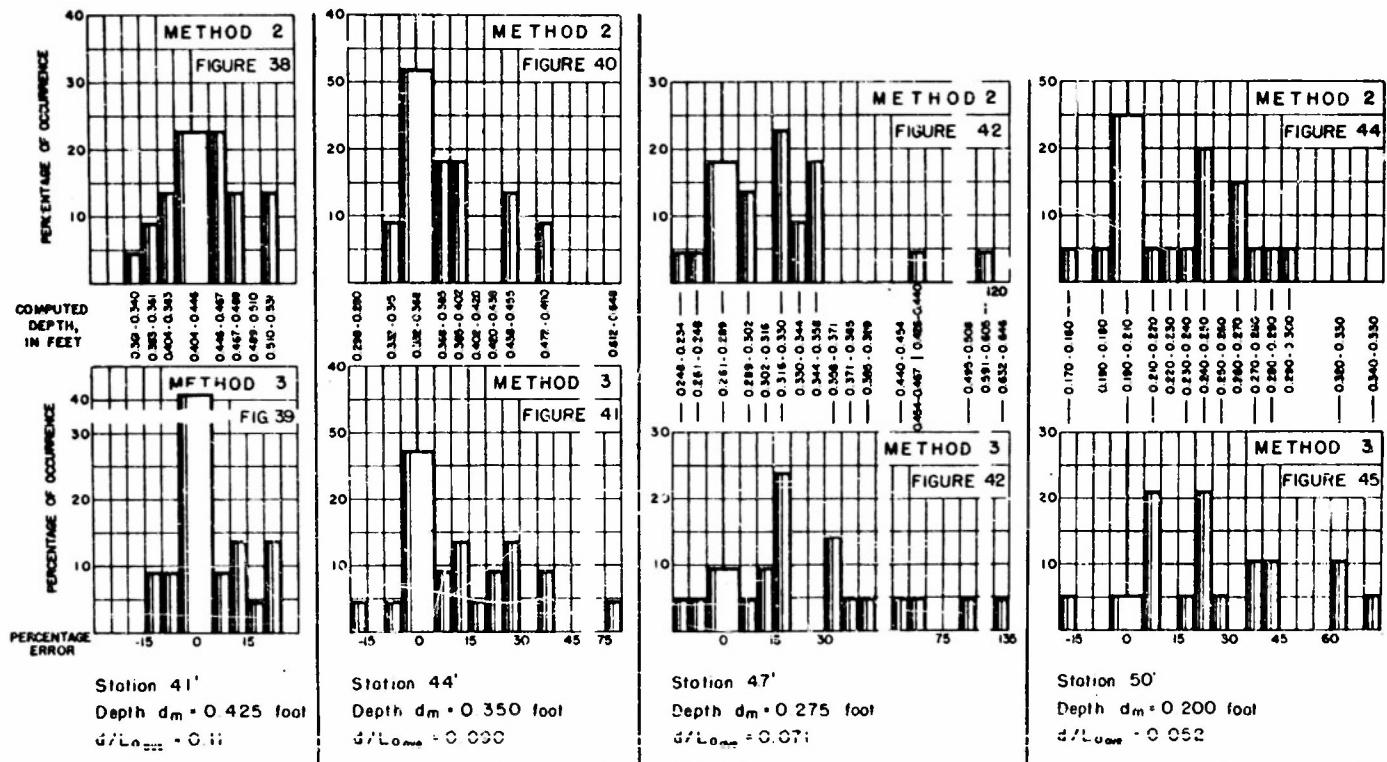
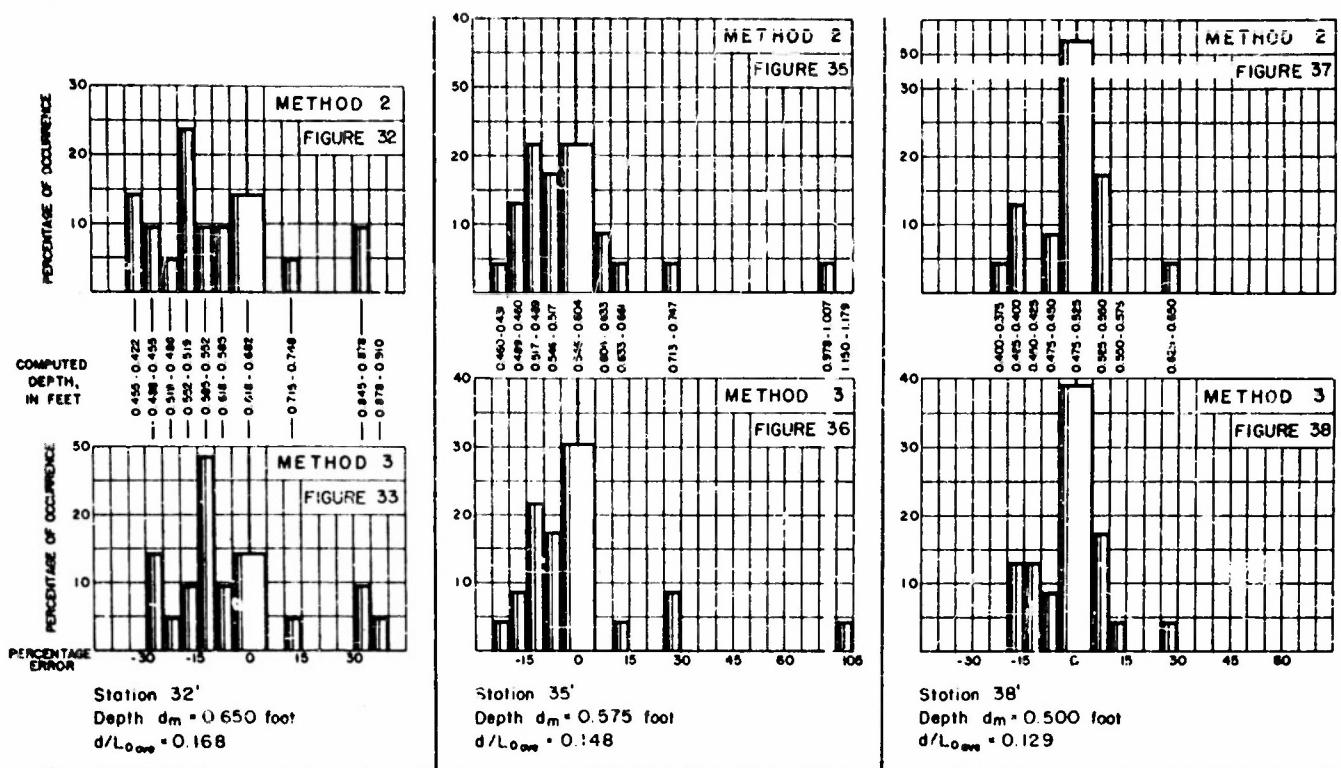
WAVE TRAVEL DIAGRAM

NON-UNIFORM WAVES ON SLOPING (1:40) BEACH



HYD-6768

FIGURE 31



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